



Application for Planning Approval

Land Use Planning and Approvals Act 1993

APPLICATION NO.

DA2026/057

LOCATION OF AFFECTED AREA

97 BRAEVIEW DRIVE, OLD BEACH

DESCRIPTION OF DEVELOPMENT PROPOSAL

OUTBUILDING

A COPY OF THE DEVELOPMENT APPLICATION MAY BE VIEWED AT www.brighton.tas.gov.au AND AT THE COUNCIL OFFICES, 1 TIVOLI ROAD, OLD BEACH, BETWEEN 8:15 A.M. AND 4:45 P.M, MONDAY TO FRIDAY OR VIA THE QR CODE BELOW. ANY PERSON MAY MAKE WRITTEN REPRESENTATIONS IN ACCORDANCE WITH S.57(5) OF THE LAND USE PLANNING AND APPROVALS ACT 1993 CONCERNING THIS APPLICATION UNTIL 4:45 P.M. ON **23/06/2026**. ADDRESSED TO THE CHIEF EXECUTIVE OFFICER AT 1 TIVOLI ROAD, OLD BEACH, 7017 OR BY EMAIL AT development@brighton.tas.gov.au. REPRESENTATIONS SHOULD INCLUDE A DAYTIME TELEPHONE NUMBER TO ALLOW COUNCIL OFFICERS TO DISCUSS, IF NECESSARY, ANY MATTERS RAISED.

JAMES DRYBURGH
Chief Executive Officer

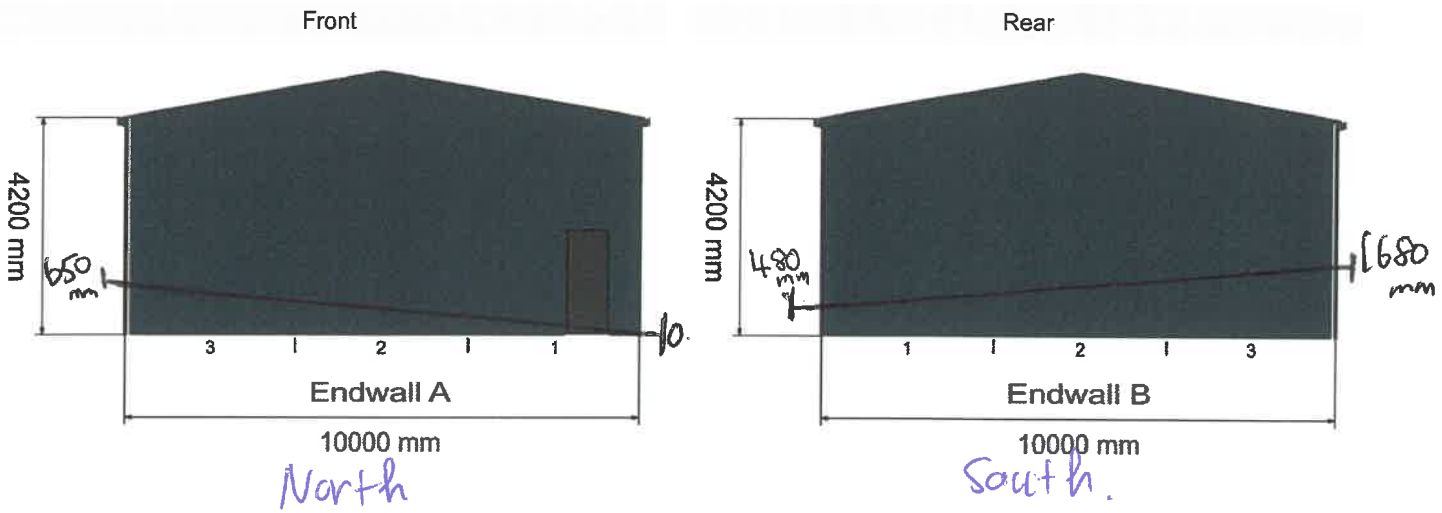


Brighton
going places

AERIAL IMAGERY

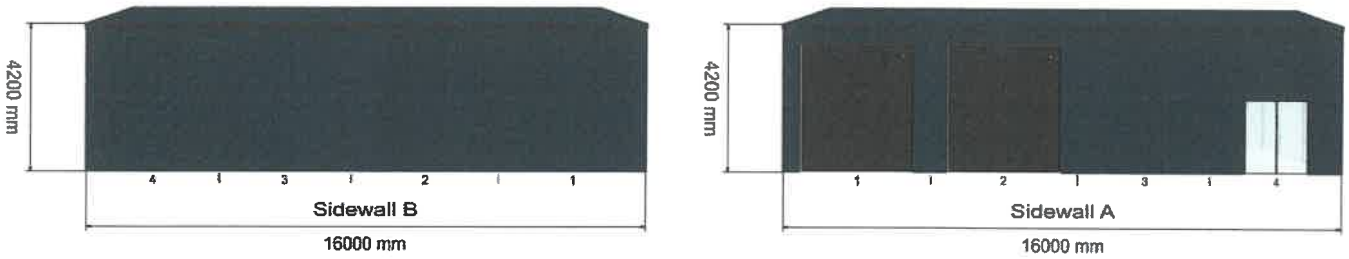


Building Specification – Drawings

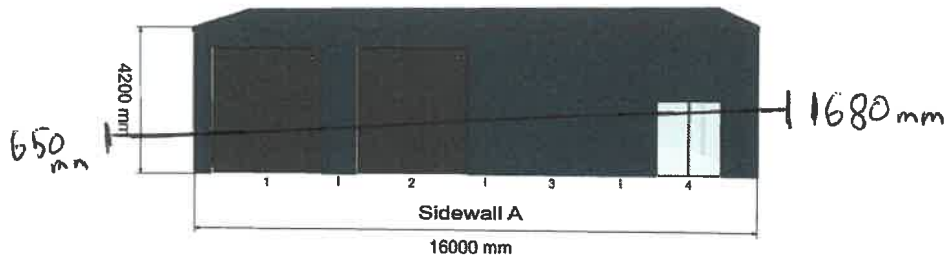


Left Side

Right Side



Western Elevation



Elevations

AS2870:2011 SITE ASSESSMENT

97 Braeview Drive

Old Beach

May 2026



GEO-ENVIRONMENTAL

S O L U T I O N S

Disclaimer: The author does not warrant the information contained in this document is free from errors or omissions. The author shall not in any way be liable for any loss, damage or injury suffered by the User consequent upon, or incidental to, the existence of errors in the information.

Investigation Details

Client:	Jack Bloomfield
Site Address:	97 Braeview Drive, Old Beach
Date of Inspection:	14/05/2026
Proposed Works:	Alterations/Additions
Investigation Method:	Geoprobe 540UD - Direct Push
Inspected by:	A. Plummer

Site Details

Certificate of Title (CT):	162272/24
Title Area:	Approx. 9946 m ²
Applicable Planning Overlays:	Bushfire-prone areas
Slope & Aspect:	5° NW facing slope
Vegetation:	Grass & Weeds

Background Information

Geology Map:	MRT
Geological Unit:	Triassic Sandstone
Climate:	Annual rainfall 600mm
Water Connection:	Tank
Sewer Connection:	Unserviced-On-site required
Testing and Classification:	AS2870:2011, AS1726:2017 & AS4055:2021

Investigation

A number of bore holes were completed to identify the distribution and variation of the soil materials at the site, bore hole locations are indicated on the site plan. See soil profile conditions presented below. Tests were conducted across the site to obtain bearing capacities of the material at the time of this investigation.

Soil Profile Summary

BH 1 Depth (m)	Cutting Depth (m)	USCS	Description
	0.00-0.10	SM	Silty SAND: brown, slightly moist, medium dense
0.00-0.20	0.10-0.70	CL	Sandy CLAY: medium plasticity, brown, slightly moist, stiff
0.20-0.25	0.70-1.00	GW	Sandy GRAVEL: yellow, brown, slightly moist, dense, refusal on rock/boulder.

Site Notes

Soils on the site are developing from Triassic sandstone, the clay fraction due to soil depth is likely to show slight ground surface movement with moisture fluctuations.

Site Classification

The site has been assessed and classified in accordance with AS2870:2011 “Residential Slabs and Footings”.

The site has been classified as:

Class S

y_s range: **0-20mm**

Notes: that is a slightly reactive site.

Wind Loading Classification

According to “AS4055:2021 - Wind Loads for Housing” the house site is classified below:

Wind Classification:	N3
Region:	A
Terrain Category:	2.5
Shielding Classification:	PS
Topographic Classification:	T2
Wind Classification:	N3
Design Wind Gust Speed – m/s ($V_{h,u}$):	50

Construction Notes & Recommendations

The site has been classified as **Class S** - Slightly reactive site, which may experience only slight ground movement from moisture changes.

It is recommended the foundations be placed on the underlying bedrock to minimise the potential for significant foundation movement.

All earthworks on site must comply with AS3798:2007, and I further recommend that consideration be given to drainage and sediment control on site during and after construction. Care should also be taken to ensure there is adequate drainage in the construction area to avoid the potential for weak bearing and foundation settlement associated with excessive soil moisture.

I also recommend that during construction that I and/or the design engineer be notified of any major variation to the foundation conditions as predicted in this report.



Dr John Paul Cumming B.Agr.Sc (hons) PhD CPSS GAICD
Director

Explanatory Notes

1 Scope of Works

The methods of description and classification of soils used in this report are based largely on Australian Standard 1726 – Geotechnical Site Investigations (AS1726:2017), with reference to Australian Standard 1289 – Methods for testing soils for engineering purposes (AS1289), for eventual Site Classification according to Australian Standard 2870 (AS2870:2011) – Residential Slabs and Footings and Australian Standard 1547 (AS1547:2012) On-site domestic wastewater management.

1.1 Site Classification AS2870:2011

Site classification with reference to the above Australian Standards are based on site reactivity.

Class	Foundation Conditions	Characteristic Surface Movement
A	Most sand and rock sites with little or no ground movement from moisture changes.	0mm
S	Slightly reactive clay sites, which may experience only slight ground movement from moisture changes.	0 – 20mm
M	Moderately reactive clay or silt sites, which may experience moderate ground movement from moisture changes.	20 – 40mm
H-1	Highly reactive clay sites, which may experience high ground movement from moisture changes.	40 – 60mm
H-2	Highly reactive clay sites, which may experience very high ground movement from moisture changes.	60 – 75mm
E	Extremely reactive sites, which may experience extreme ground movement from moisture changes.	>75mm

*Note: Soils where foundation performance may be significantly affected by factors other than reactive soil movement are classified as **Class P**.*

A site is classified as **Class P** when:

- The bearing capacity of the soil profile in the foundation zone is generally less than 100kpa
- If excessive foundation settlement may occur due to loading on the foundation.
- The site contains uncontrolled fill greater than 0.8m in depth for sandy sites and 0.4m in depth for other soil materials.
- The site is subject to mine subsistence, landslip, collapse activity or coastal erosion.
- The site is underlain by highly dispersive soils with significant potential for erosion
- If the site is subject to abnormal moisture conditions which can affect foundation performance

1.2 Soil Characterisation

This information explains the terms of phrase used within the soil description area of the report.

It includes terminology for cohesive and non-cohesive soils and includes information on how the Unified Soil Classification Scheme (USCS) codes are determined.

NON COHSIVE – SAND & GRAVEL		
Consistency Description	Field Test	Dynamic Cone Penetrometer blows/100 mm
Very loose (VL)	Easily penetrated with 13 mm reinforcing rod pushed by hand.	0 - 1
Loose (L)	Easily penetrated with 13 mm reinforcing rod pushed by hand. Can be excavated with a spade; 50 mm wooden peg can be easily driven.	1 - 3
Medium dense (MD)	Penetrated 300 mm with 13 mm reinforcing rod driven with 2 kg hammer, - hard shovelling.	3 - 8
Dense (D)	Penetrated 300 mm with 13 mm reinforcing rod driven with 2 kg hammer, requires pick for excavation; 50 mm wooden peg hard to drive.	8 - 15
Very dense (VD)	Penetrated only 25 - 50 mm with 13 mm reinforcing rod driven with 2 kg hammer.	>15

COHESIVE - SILT & CLAY		
Consistency Description	Field Test	Indicative undrained shear strength kPa
Very soft	Easily penetrated >40 mm by thumb. Exudes between thumb and fingers when squeezed in hand.	<12
Soft	Easily penetrated 10 mm by thumb. Moulded by light finger pressure	>12 and <25
Firm	Impression by thumb with moderate effort. Moulded by strong finger pressure	>25 and <50
Stiff	Slight impression by thumb cannot be moulded with finger.	>50 and <100
Very Stiff	Very tough. Readily indented by thumbnail.	>100 and <200
Hard	Brittle. Indented with difficulty by thumbnail.	>200

1.3 USCS Material Descriptions

Soils for engineering purposes are the unconsolidated materials above bedrock, they can be residual, alluvial, colluvial or aeolian in origin.

Major Divisions	Particle size mm	USCS Group Symbol	Typical Names	Laboratory Classification					
				% < 0.075 mm (2)	Plasticity of fine fraction	$C_u = \frac{D_{60}}{D_{10}}$	$C_c = \frac{(D_{30})^2}{(D_{10})(D_{60})}$	NOTES	
COARSE GRAINED SOILS (more than half of material less than 63 mm & larger than 0.075 mm)	BOULDERS _____ 200								
	COBBLES _____ 63								
	GRAVELS (more than half of coarse fraction is larger than 2.36 mm)	coarse _____ 20	GW	Well graded gravels and gravel-sand mixtures, little or no fines	0-5	—	>4	Between 1 and 3	(1) Identify fines by the method given for fine-grained soils. (2) Borderline classifications occur when the percentage of fines (fraction smaller than 0.075 mm size) is greater than 5% and less than 12%. Borderline classifications require the use of SP-SM, GW-GC.
		medium _____ 6	GP	Poorly graded gravels and gravel-sand mixtures, little or no fines, uniform gravels	0-5	—	Fails to comply with above		
		fine _____ 2.36	GM	Silty gravels, gravel-sand-silt mixtures (1)	12-50	Below 'A' line or PI<4	—	—	
			GC	Clayey gravels, gravel-sand-clay mixtures (1)	12-50	Above 'A' line and PI>7	—	—	
	SANDS (more than half of coarse fraction is smaller than 2.36 mm)	coarse _____ 0.6	SW	Well graded sands and gravelly sands, little or no fines	0-5	—	>6	Between 1 and 3	
		medium _____ 0.2	SP	Poorly graded sands and gravelly sands, little or no fines	0-5	—	Fails to comply with above		
		fine 0.075	SM	Silty sands, sand silt mixtures (1)	12-50	Below 'A' line or PI<4	—	—	
			SC	Clayey sands, sand-clay mixtures (1)	12-50	Above 'A' line and PI>7	—	—	
	FINE GRAINED SOILS (more than half of material less than 63 mm & smaller than 0.075 mm)	SILTS & CLAYS (Liquid Limit ≤50%)	ML	Inorganic silts, very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity					
			CL CI	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays					
CL			Organic silts and clays of low plasticity						
SILTS & CLAYS (Liquid Limit >50%)		MH	Inorganic silts, micaceous or diatomaceous fine sands or silts, elastic silts						
		CH	Inorganic clays of high plasticity, fat clays						
		OH	Organic silts and clays of high plasticity						
HIGHLY ORGANIC SOILS		PT	Peat and other highly organic soils						

Use the gradation curve of material passing 63 mm for classification of fractions according to the criteria given in 'Major Divisions'

Plasticity Chart

For classification of fine grained soils and fine fraction of coarse grained soils.

The Plasticity Chart is a graph with Plastic Index (%) on the vertical axis (0 to 60) and Liquid Limit (%) on the horizontal axis (0 to 100). It features several classification boundaries: a horizontal line at PI=0, a diagonal line labeled 'U-line' (PI = LL - 0.73(LL - 20)), a diagonal line labeled 'A-line' (PI = 0.73(LL - 20)), and a diagonal line labeled 'I-line' (PI = 0.009(LL - 8)). Vertical lines at LL=40, 50, and 60 are labeled 'Low', 'Medium', and 'High' plasticity respectively. Classification regions include: CL (low plasticity clay), CH (high plasticity clay), ML (low plasticity silt), MH (high plasticity silt), OL (organic low plasticity clay), OH (organic high plasticity clay), MI & CI (intermediate plasticity), and ML & OL (lean silts and clays).

Grain size analysis is performed by two processes depending on particle size. Sand silt and clay particles are assessed using a standardised hydrometer test, and coarse sand and larger is assessed through sieving by USCS certified sieves. For more detail see the following section.

Soil Classification	Particle Size
Clay	Less than 0.002mm
Silt	0.002 – 0.06mm
Fine/Medium Sand	0.06 – 2.0mm
Coarse Sand	2.0mm – 4.75mm
Gravel	4.75mm – 60.00mm

1.4 Bearing Capacities and DCP testing.

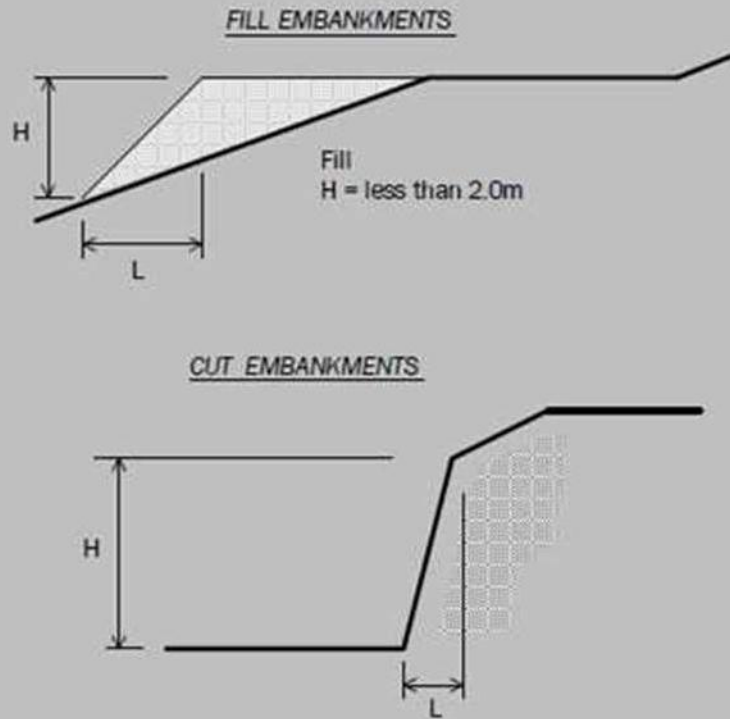
DCP and PSP weighted penetrometer tests – Dynamic Cone Penetrometer (DCP) and Perth Sand Penetrometer (PSP) tests are carried out by driving a rod into the ground with a falling weight hammer and measuring the blows for successive 100mm increments of penetration. Normally, there is a depth limitation of 1.2m but this may be extended in certain conditions by the use of extension rods. The methods for the two tests are quite similar.

- Dynamic Cone Penetrometer – a 16mm rod with a 20mm diameter cone end is driven with a 9kg hammer dropping 510mm (AS 1289, Test 6.3.2).
- Perth Sand Penetrometer – a 16mm diameter flat-ended rod is driven with a 9kg hammer, dropping 600mm (AS 1289 Test 6.3.3). This test was developed for testing the density of sands and is mainly used in granular soils and filling.

Site Anomalies – During construction GES will need to be notified of any major variation to the foundation conditions as predicted in this report.

1.5 Batter Angles for Embankments (Guide Only)

Note : Retaining walls or other form of soil retaining methods must be adopted where the slope ratio is greater than that indicated in the table below :-



MATERIAL TYPE (refer soils report)		EMBANKMENT SLOPES (Height : Length)	
		Compacted Fill	Cutting
Stable Rock (A*)		2 : 3	6 : 1
Sand (A*)		1 : 2	2 : 3
Silt (P*)		1 : 4	1 : 4
Clay	Firm Clay	1 : 2	1 : 1
	Soft Clay	Not Suitable	2 : 3
Soft Soils (P*)		Not Suitable	Not Suitable

Glossary of Terms

Bearing Capacity – Maximum bearing pressure that can be sustained by the foundation from the proposed footing system under service loads which should avoid failure or excessive settlement.

Clay – (Mineral particles less than 0.002mm in diameter). Fine grained cohesive soil with plastic properties when wet. Also includes sandy clays, silty clays, and gravelly clays.

Dynamic Cone Penetrometer (DCP) – Field equipment used to determine underlying soil strength and therefore bearing capacity (kPa) by measuring the penetration of the device into the soil after each hammer blow.

Dispersive soil – A soil that has the ability to pass rapidly into suspension in water.

Footing – Construction which transfers the load from the building to the foundation.

Foundation – Ground which supports the building

Landslip – Foundation condition on a sloping site where downhill foundation movement or failure is a design consideration.

Qualified Engineer – A professional engineer with academic qualifications in geotechnical or structural engineering who also has extensive experience in the design of the footing systems for houses or similar structures.

Reactive Site – Site consisting of clay soil which swells on wetting and shrinks on drying by an amount that can damage buildings on light strip footings or unstiffened slabs. Includes sites classified as S, M, H-1, H-2 & E in accordance with AS2870-2011.

Sand – (Mineral particles greater than 0.02mm in diameter). Granular non-cohesive, non-plastic soil that may contain fines including silt or clay up to 15%.

Services – Means all underground services to the site including but not limited to power, telephone, sewerage, water & storm water.

Silt – (Mineral particles 0.002 – 0.02mm in diameter). Fine grained non-cohesive soil, non-plastic when wet. Often confers a silky smoothness of field texture, regularly includes clay and sand to form clayey silts, sandy silts and gravelly silts.

Site – The site title, as denoted by address, lot number, or Certificate of Title (CT) number, or Property Identification Number (PID).

Surface Movement (Ys) – Design movement (mm) at the surface of a reactive site caused by moisture changes.

Disclaimer

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The scope of this study does not allow for the review of every possible geotechnical parameter or the soil conditions over the whole area of the site. Soil and rock samples collected from the investigation area are assumed to be representative of the areas from where they were collected and not indicative of the entire site. The conclusions discussed within this report are based on observations and/or testing at these investigation points.

This report does not purport to provide legal advice. Readers of the report should engage professional legal practitioners for this purpose as required.

No responsibility is accepted for use of any part of this report in any other context or for any other purpose by a third party.

Site Plan



CERTIFICATE OF QUALIFIED PERSON – ASSESSABLE ITEM

Section 321

Form **55**

To: Owner /Agent
 Address
 Suburb/postcode

Qualified person details:

Qualified person:
Address: Phone No:
 Fax No:
Licence No: Email address:

Qualifications and Insurance details: (description from Column 3 of the Director's Determination - Certificates by Qualified Persons for Assessable Items)

Speciality area of expertise: (description from Column 4 of the Director's Determination - Certificates by Qualified Persons for Assessable Items)

Details of work:

Address: Lot No:
 Certificate of title No:

The assessable item related to this certificate: (description of the assessable item being certified)
Assessable item includes –
- a material;
- a design
- a form of construction
- a document
- testing of a component, building system or plumbing system
- an inspection, or assessment, performed

Certificate details:

Certificate type: (description from Column 1 of Schedule 1 of the Director's Determination - Certificates by Qualified Persons for Assessable Items n)

This certificate is in relation to the above assessable item, at any stage, as part of - (tick one)

building work, plumbing work or plumbing installation or demolition work
or

a building, temporary structure or plumbing installation:

In issuing this certificate the following matters are relevant –

Documents:	The attached soil report for the address detailed above in 'details of work'
Relevant calculations:	Reference the above report.
References:	AS2870:2011 residential slabs and footings AS1726:2017 Geotechnical site investigations CSIRO Building technology file – 18.

Substance of Certificate: (what it is that is being certified)

Site Classification consistent with AS2870-2011.
--

Scope and/or Limitations

The classification applies to the site as inspected and does not account for future alteration to foundation conditions as a result of earth works, drainage condition changes or variations in site maintenance.

I, John-Paul Cumming certify the matters described in this certificate.

Qualified person:	<i>Signed:</i>	<i>Certificate No:</i>	<i>Date:</i>
		J13135	19/05/2026



DISPERSIVE SOIL ASSESSMENT

97 Braeview Drive

Old Beach

May 2026



GEO-ENVIRONMENTAL

S O L U T I O N S

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Investigation Details

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Site Address:	97 Braeview Drive, Old Beach
Date of Inspection:	14/05/2026
Proposed Works:	Alterations/Additions
Investigation Method:	Geoprobe 540UD - Direct Push
Inspected by:	A. Plummer

Site Details

Certificate of Title (CT):	162272/24
Title Area:	Approx. 9946 m ²
Applicable Planning Overlays:	Bushfire-prone areas
Slope & Aspect:	5° NW facing slope
Vegetation:	Grass & Weeds

Background Information

Geology Map:	MRT
Geological Unit:	Triassic Sandstone
Climate:	Annual rainfall 600mm
Water Connection:	Tank
Sewer Connection:	Unserviced-On-site required
Testing and Classification:	AS2870:2011, AS1726:2017 & AS4055:2021

Investigation

A number of bore holes were completed to identify the distribution and variation of the soil materials at the site, bore hole locations are indicated on the site plan. See soil profile conditions presented below. Tests were conducted across the site to obtain bearing capacities of the material at the time of this investigation.

Soil Profile Summary

BH 1 Depth (m)	Cutting Depth (m)	USCS	Description
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0.20-0.25	0.70-1.00	GW	Sandy GRAVEL: yellow, brown, slightly moist, dense, refusal on rock/boulder.

Site Notes

The soil on site has formed from Triassic sandstone. The subsoil was tested for dispersion using the Emerson Test and was found to be slightly dispersive Class 2(2) -obvious milkiness, <50% of the aggregate affected.

Dispersive Soil Assessment

The dispersive soil assessment of the property considers the proposed construction area.

Potential for dispersive soils

The site has been identified as an area subject to a tunnel erosion hazard according to 'Dispersive Soils and Their Management: Technical Reference Manual'. This is due to the soils present on site that developed from Triassic Sandstone that contain considerable fine sand/silt content and medium plastic clays. Triassic Sandstone in the local area is known to produce soils with an excess of sodium on the soil exchange complex, which can cause soil dispersion. Under some circumstances the presence of dispersive soils can also lead to significant erosion, and in particular tunnel erosion. Based upon field survey of the property, no visible tunnel or gully erosion was identified. However, a soil sampling program was undertaken to identify the presence of dispersive soils in the proposed development areas.

Soil sampling and testing

Samples were taken at the site for assessment of dispersion. An Emerson (1968) Dispersion test was conducted to determine if these samples were dispersive.

The soil samples taken from site were found to be Slightly dispersive Class 2(2) -obvious milkiness, <50% of the aggregate affected.

Management Recommendations

A number of site and soil management measures are recommended for development on the site.

The proposed site cut/fill and driveway areas must be managed by:

- Applying a geo-fabric, jute mesh or similar material to the exposed batters of any cutting on site and revegetating the slope
- Applying a surface layer of at least 50mm of suitable crushed rock/gravel to the driveway surface (and any proposed house pad), with adequate compaction to ensure a relatively impervious surface to maintain site surface stability
- Vegetation on any fill batters must be established and maintained, if any bare area of soil on the batter develops then it must be top-dressed with suitable topsoil and additional vegetation planted

The risk of erosion and tunnel erosion associated with construction must be minimized by:

- Any new water, power, or other service trenches within the property must ensure recommendations for dispersive soils are followed:
 - o Where possible trenches to be placed shallow in topsoil and mounded over to achieve the required cover depth
 - o If buried the trench must be backfilled in layers of no more than 200mm with clay with 5% by weight gypsum added (the clay must be sufficiently moist to allow good compaction).
 - o The trench must be finished with at least 150mm depth of non-dispersive suitable topsoil and finished to a level at least 75mm above natural ground to allow for possible settlement
- Vegetation cover must be maintained wherever possible on the property
- Foundations may be placed into the natural soil; however, care must be taken to ensure all exposed soil in the foundation area is compacted and 1Kg/m² of gypsum is applied. Excavated fill from the construction area is not recommended for reuse on site in landscaping unless it is appropriately treated with gypsum, compacted, and capped with topsoil with natural soil and gypsum
- All stormwater runoff from the dwelling to be directed to mains connection (all the drains are to be adequately treated with gypsum)
- Drainage of any site cut must not employ conventional rock drain construction; it must adhere to recommendations for dispersive soils (unless founded entirely in rock)
- All excavation works on site should be monitored for signs of soil dispersion and remedial action taken as required – any excavated fill from the construction area is not recommended for reuse on site in landscaping unless it is appropriately treated with gypsum, compacted, and capped with topsoil

BRI-S7.7 Development Standards for Buildings and Works

Acceptable Solutions	Comment
<p>A1 Development must be for:</p> <p>(a) works not involving the release of concentrated water or the disturbance of soils;</p> <p>(b) additions or alterations to an existing building, or the construction of a non-habitable building, provided the development area is no more than 100m²; or</p> <p>(c) forestry operations in accordance with a certified Forest Practices Plan.</p>	<p>Non-compliance See P1 below</p>

Performance Criteria	Comment
<p>P1 Development must be designed, sited and constructed to minimise the risks associated with dispersive soil to property and the environment having regard to:</p> <p>(a) the dispersive potential of soils in the vicinity of proposed buildings, driveways, services and the development area generally;</p> <p>(b) the potential of the development to affect or be affected by erosion, including gully and tunnel erosion;</p> <p>(c) the dispersive potential of soils in the vicinity of water drainage lines, infiltration areas and trenches, water storages, ponds, dams and disposal areas;</p> <p>(d) the level of risk and potential consequences for property and the environment from potential erosion, including gully and tunnel erosion;</p> <p>(e) management measures that would reduce risk to an acceptable level; and</p> <p>(f) the advice contained in a dispersive soil management plan.</p>	<p>The site has returned a slightly dispersive result for soil dispersion. Provided the management recommendations are followed there is acceptably low risk of the property and environment being impacted by soil erosion resulting from dispersive soils.</p>


The development is considered to be consistent with the objective of this specific area plan.

Conclusions

There is a low risk associated with dispersive soils and potential erosion on the site provided the recommendations in this report are adhered to. Efforts should be made to cover all exposed soils on cut/fill batters with topsoil and seeded with well suited pasture species to avoid rainwater, runoff, surface water flows from intercepting exposed subsoils.

A number of site management recommendations have been made in this report and further information can also be found in the publication “Dispersive soils and their management – Technical manual” (DPIWE Tas 2009)

It is recommended that during construction that GES be notified of any variation to the soil conditions as outlined in this report.



Dr John Paul Cumming B.Agr.Sc (hons) PhD CPSS GAICD
Environmental and Engineering Soil Scientist

Appendix 1– Soil test results

Laboratory Test Results

Sample Submitted By: A Plummer
Date Submitted: 14/05/2026
Sample Identification: 2 samples – 97 Braeview Drive, Honeywood
Soil to be tested: Emerson soil dispersion test

Result:

Sample	Texture	Emerson class	Description
Sample 1	clay	Class 2 (2)	Some dispersion <50% affected
Sample 2	clay	Class 2 (2)	Some dispersion <50% affected

Notes: Some dispersion with obvious milkiness affecting <50% of the aggregate.

Sample Tested by: A Plummer

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