



Application for Planning Approval

Land Use Planning and Approvals Act 1993

APPLICATION NO.

DA2024/173

LOCATION OF AFFECTED AREA

28 CROOKED BILLET DRIVE, BRIDGEWATER

DESCRIPTION OF DEVELOPMENT PROPOSAL

PAVEMENT AND STORMWATER MANAGEMENT UPGRADE & NEW UNDERGROUND STORAGE TANK

A COPY OF THE DEVELOPMENT APPLICATION MAY BE VIEWED AT www.brighton.tas.gov.au AND AT THE COUNCIL OFFICES, 1 TIVOLI ROAD, OLD BEACH, BETWEEN 8:15 A.M. AND 4:45 P.M, MONDAY TO FRIDAY OR VIA THE QR CODE BELOW. ANY PERSON MAY MAKE WRITTEN REPRESENTATIONS IN ACCORDANCE WITH S.57(5) OF THE LAND USE PLANNING AND APPROVALS ACT 1993 CONCERNING THIS APPLICATION UNTIL 4:45 P.M. ON **08/09/2025**. ADDRESSED TO THE CHIEF EXECUTIVE OFFICER AT 1 TIVOLI ROAD, OLD BEACH, 7017 OR BY EMAIL AT development@brighton.tas.gov.au. REPRESENTATIONS SHOULD INCLUDE A DAYTIME TELEPHONE NUMBER TO ALLOW COUNCIL OFFICERS TO DISCUSS, IF NECESSARY, ANY MATTERS RAISED.

JAMES DRYBURGH
Chief Executive Officer



Brighton
going places

CIVIL ENGINEERING DRAWINGS



Email: pda.hbt@pda.com.au

[illegible]

Sheet 1 of 1

1. NO ATTEMPT HAS BEEN MADE TO LOCATE ALL SERVICES. ONLY THOSE SERVICES CONSPICUOUS DURING FIELD SURVEYS ARE SHOWN. PRIOR TO ANY DEMOLITION, EXCAVATION OR CONSTRUCTION ON THE SITE, THE RELEVANT AUTHORITY(S) SHOULD BE CONTACTED FOR POSSIBLE LOCATION OF FURTHER UNDERGROUND SERVICE AND DETAILED LOCATIONS OF ALL SERVICES.

3. REFER IPWEA - LGAT SPECIFICATIONS, TASMANIAN STANDARD DRAWINGS
ISSUED - 03 DECEMBER 2020

TSD-G01.v3 - TRENCH REINSTATEMENT FLEXIBLE PAVEMENTS
TSD-G02.v3 - URBAN ROADS - TYPICAL SERVICE LOCATIONS
TSD-R01.v3 - RURAL ROADS UNSEALED
TSD-R02.v3 - RURAL ROADS SEALED
TSD-R03.v3 - RURAL ROADS - TYPICAL DRIVEWAY ACCESS
TSD-R04.v3 - RURAL ROADS - TYPICAL DRIVEWAY PROFILE
TSD-R05.v3 - TRUCK ACCESS TO RURAL PROPERTIES 'TYPE A'
TSD-R06.v3 - URBAN ROADS - TYPICAL SECTIONS & PAVEMENT WIDTHS
TSD-R09.v3 - URBAN ROADS - DRIVEWAYS
TSD-R11.v3 - URBAN ROADS - FOOTPATHS
TSD-R12.v3 - SUB SOIL DRAINS - CONSTRUCTION DETAILS
TSD-R13.v3 - SUBSOIL DRAINS PIT CONNECTION - TYPE FD
TSD-R14.v3 - CONCRETE KERBS & CHANNELS DIMENSIONS
TSD-R15.v3 - CONCRETE KERBS & CHANNELS CONSTRUCTION DETAILS
TSD-R16.v3 - CONCRETE KERBS & CHANNELS VEHICULAR CROSSINGS
TSD-R18.v3 - CONCRETE KERBS & CHANNELS ACCESS RAMPS
TSD-R26.v3 - DELINEATORS
TSD-R28.v3 - W-BEAM - INSTALLATION DETAILS
TSD-SW01.v3 - PIPE INSTALLATION ANCHOR BLOCKS
TSD-SW02.v3 - MANHOLES - 100-600 DIA. PIPES - GENERAL ARRANGEMENTS
TSD-SW03.v3 - MANHOLES - 100-600 DIA. PIPES - BENCHING DETAILS
TSD-SW09.v3 - SIDE ENTRY PITS - 'TYPE 3'
TSD-SW11.v3 - SIDE ENTRY PITS - KERB TRANSITIONS
TSD-SW15.v3 - STORMWATER - 'GP'
TSD-SW17.v3 - OUTLET HEADWALLS 300 - 600 DIA PIPES
TSD-SW25.v3 - STORMWATER PROPERTY CONNECTIONS TO MAINS
TSD-SW26.v3 - SADDLE CONNECTION TO STORMWATER DRAIN
TSD-SW27.v3 - REPAIRS/ NEW CONNECTION TO STORMWATER DRAIN
TSD-SW28.v3 - GUIDELINES FOR SEDIMENT CONTROL
TSD-RF04.v3 - NATURE STRIP DETAILS

4. CONSTRUCTION TO COMPLY WITH WSAA SEWERAGE CODE OF AUSTRALIA (MELBOURNE RETAIL WATER AGENCIES EDITION) - WSA 02-2014-3.1 v3 AND TASWATER SUPPLEMENT TO THE CODE.

5. ALL WORKS TO BE CONSTRUCTED IN ACCORDANCE WITH THE TASMANIAN SUBDIVISION GUIDELINES (VERSION 1.0 , DATED OCTOBER 2013) UNLESS OTHERWISE NOTED.

6. ALL CONNECTIONS TO EXISTING SEWER AND WATER MAINS TO BE CARRIED OUT BY TASWATER OR APPROVED CONTRACTOR AT DEVELOPER'S COST UNLESS APPROVED OTHERWISE.

1. DATUM:	GDA 2020 & AHD
2. PLANAR OR GRID:	PLANAR
3. SITE CONTROL:	SPM 8324
4. SURVEYED BY:	PDA SURVEYORS
5. ACCURACY OF SURVEY:	0.02m
6. ACCURACY/STATUS OF EXISTING & FUTURE PROPERTY BOUNDARIES:	0.02m

1. ALL STORMWATER LOT CONNECTIONS TO BE DN150 UNLESS OTHERWISE NOTED.
2. ALL STORMWATER LINES DN300 PVC-U SN8 OR EQUIVALENT UNLESS NOTED OTHERWISE.
3. ALL PRIVATE STORMWATER LINES TO BE INSTALLED TO AS3500.3
4. ALL FOOTPATHS TO BE N25 CONCRETE 100mm THICK, SL72 CENTRAL, 100 x 20 FCR BASE.
5. 2 x 65 DIA WEEPHOLES TO BE PLACED IN ALL STORMWATER SIDE ENTRY PITS AND MANHOLES WITH NO SUBSOIL DRAIN CONNECTIONS.
6. ALL STORMWATER BRANCHES TO TERMINATE AT PROPERTY BOUNDARIES WITH AN INSPECTION OPENING RAISED TO SURFACE WITH AN APPROVED COVER 1m INSIDE PROPERTY BOUNDARY.
7. ALL LOT CONNECTIONS TO BE CONSTRUCTED IN THE POSITION SHOWN UNLESS APPROVED BY THE SUPERINTENDENT.
8. FINAL POSITION AND WIDTH OF ALL DRAINAGE EASEMENTS IS TO BE DETERMINED FOLLOWING CONSTRUCTION.
9. EXTENT OF ROADWORKS SHOWN SHADED.
10. ALL FILL AREAS TO BE PLACED AND COMPACTED PRIOR TO TRENCH EXCAVATION.
11. PROVIDE TRAFFICABLE LIDS TO CONNECTIONS IN TRAFFICABLE AREAS INCLUDING DRIVEWAYS.
12. ALL DRIVEWAYS TO HAVE MAXIMUM GRADE 20% OUTSIDE LOT BOUNDARIES AND 25% INSIDE LOT BOUNDARIES AND CONSTRUCTED AS PER TSD-R09 V3.
13. PROVIDE MINIMUM 150mm CLEARANCE FROM TOP OF ROD EYES TO SURFACE COVER LID.

1. ALL EXCAVATION OVER 1.5m IN DEPTH MUST BE CARRIED OUT IN ACCORDANCE WITH WORKPLACE SAFETY STANDARDS CODE OF PRACTICE FOR EXCAVATION WORK.
2. COMPACTED FCR BACKFILL UNDER ROADS & DRIVEWAYS.
3. ALL ANCHOR BLOCKS (CONCRETE BULKHEADS) ARE TO BE KEYED INTO UNDISTURBED, COMPETENT MATERIAL TO ENSURE THE MIGRATION OF BEDDING AND BACKFILL MATERIAL IS REDUCED AND THE INTEGRITY OF THE PIPE IS MAINTAINED.
4. TRENCHES >10% ARE TO HAVE CONTINUOUS DRAINAGE PATH INSTALLED TO ENSURE WATER THAT HAS MIGRATED INTO THE TRENCH IS CARRIED TO THE RETICULATED SYSTEM. REFER TSD-SW01.
5. ALL FILL TO BE PLACED & COMPACTED PRIOR TO TRENCH EXCAVATION.
6. FILL IN EXCESS OF 300mm DEPTH TO BE COMPACTED TO 95% STANDARD DENSITY (AS 1289E4.01) IN 150mm MAX LAYERS.
7. PROVIDE MINIMUM 150mm CLEARANCE FROM TOP OF ROD EYES TO SURFACE COVER LID.


1. APPLY 100mm THICK 20mm SCREENED LOAM TO NATURE STRIP & APPLY GRASS SEED AS PER TSD-RF04-v3.
2. NATURE STRIPS TO BE WATERED & MAINTAINED INCLUDING WEED REMOVAL & MOWING THROUGHOUT THE MAINTENANCE PERIOD.
3. FOR FILL BATTERS, BENCH AS SHOWN AND PLACE APPROVED FILL IN 300mm LAYERS COMPACTED TO 95% MAX DRY DENSITY. FILL TO BE PLACED & COMPACTED PRIOR TO TRENCH EXCAVATION.
4. MATERIAL TO BE PLACED AND COMPACTED UNIFORMLY IN LAYERS ACROSS THE EMBANKMENT NO GREATER THAN 300mm TO ACHIEVE 95% MDD. FOR FILL BATTERS, BENCH AS SHOWN.

1. FILL MATERIAL FOR NEW ROAD AND FILL EMBANKMENTS MUST NOT CONTAIN ORGANIC OR OTHER MATERIALS THAT DECOMPOSE OR OTHERWISE LEAD TO LONG TERM SETTLEMENT AND TO BE APPROVED BY SUPERINTENDENT BEFORE USE.
2. MATERIAL TO BE PLACED AND COMPACTED UNIFORMLY IN LAYERS UNDER NEW ROAD NO GREATER THAN 150mm TO ACHIEVE 98% MDD AND ACROSS ROAD EMBANKMENT NO GREATER THAN 300mm TO ACHIEVE 95% MDD. FOR FILL BATTERS, BENCH AS SHOWN.
3. ROAD EMBANKMENT TO BE FULLY CONSTRUCTED PRIOR TO TRENCHING FOR WATER AND SEWER SERVICES.
4. FILL TO BE PLACED & COMPACTED PRIOR TO TRENCH EXCAVATION.
5. SITE FILLING THAT EXCEEDS A DEPTH OF 300mm MUST BE PLACED IN ACCORDANCE WITH THE AS3798 GUIDELINES FOR COMMERCIAL AND RESIDENTIAL DEVELOPMENTS 1996. UPON COMPLETION OF THE WORKS THE CONTRACTORS GEOTECHNICAL ENGINEER MUST CONFIRM IN WRITING THAT THE WORKS HAVE BEEN CARRIED OUT IN ACCORDANCE WITH AS3798.

- REFER BEST PRACTICE EROSION & SEDIMENT CONTROL BOOKLET 2022 - "EROSION AND SEDIMENT CONTROL". AVAILABLE AT www.derwentestuary.org.au/publications
2. AREAS OF GROUND DISTURBANCE ARE SHOWN. WORKS TO BE CONFINED TO WITHIN THESE AREAS. CLEARING FOR WORKS TO BE LIMITED TO WITHIN 5 METRES FROM THE EDGE OF ANY ESSENTIAL CONSTRUCTION ACTIVITY. (REFER PAGES 34 & 35: MINIMISE SOIL DISTURBANCE).
3. ALL EXCAVATION IS TO COMPLY WITH " BEST PRACTICE EROSION & SEDIMENT CONTROL BOOKLET 2022 - "EROSION AND SEDIMENT CONTROL"." AVAILABLE AT www.derwentestuary.org.au/publications
4. EXCAVATION TO BE CARRIED OUT OVER A MINIMUM TIME PERIOD. TOP SOIL TO BE STOCKPILED SEPARATELY AND SPREAD OVER BACKFILLED AREAS. SOIL TO BE STOCKPILED IN A NARROW CORRIDOR ON THE UPSTREAM SIDE OF ALL EXCAVATION. TEMPORARY CATCH DRAINS TO BE CONSTRUCTED ON THE UPSTREAM SIDE OF STOCKPILES AND EXCAVATED AREAS, DIRECTING RUNOFF TO EXISTING STORMWATER SYSTEM. (REFER PAGE 47: PROTECT STOCKPILES AND SERVICE TRENCHES).
5. SEDIMENT FENCES & FIBRE ROLLS TO BE USED ON THE DOWNSTREAM SIDE OF ALL STOCKPILES AND TO EXTENTS SHOWN ON THIS DRAWING. PREVENT ENTRY OF SILT TO EXISTING STORMWATER INLETS AND WATER COURSES DURING CONSTRUCTION. (REFER PAGE 67: SEDIMENT FENCES).
6. EVERY EFFORT TO BE MADE TO MINIMIZE SPREADING SEDIMENT ON TO SEALED AREAS WHEN VEHICLES LEAVE THE SITE, INCLUDING THE WASHING DOWN OF TYRES. (REFER PAGE 59: WHEEL WASH OR RUMBLE GRID).
7. NO TOPSOIL SHALL BE REMOVED FROM LAND OUTSIDE THE AREAS OF GROUND DISTURBANCE SHOWN.
8. ALL AREAS OF GROUND DISTURBANCE TO BE RE-VEGETATED
9. ALL STOCKPILES TO BE POSITIONED CLEAR OF WATER COURSES AND TO ENSURE THAT NO SILT RUNOFF CAN ENTER WATER COURSES.
10. DURING WINDY CONDITIONS AND/OR HOT WEATHER, WET DOWN EXPOSED SOIL SLIGHTLY & REGULARLY TO PREVENT DUST NUISANCE. (REFER PAGE 53: CONTROL DUST AND LITTER).
11. PRIOR TO PRACTICAL COMPLETION OF EACH APPROVED CONSTRUCTION STAGE, ALL DISTRIBUTED SURFACES ON THE SITE, EXCEPT THE AREAS SET ASIDE FOR ROADWAYS & FOOTPATHS, MUST BE DRESSED TO A MINIMUM OF 50mm WITH:
 - APPROVED LOCAL STOCKPILED TOPSOIL
 - APPROVED WEED FREE IMPORTED TOPSOIL
 - RE-VEGETATED WITH LOCAL PLANTS & GRASSES AND STABILISED.

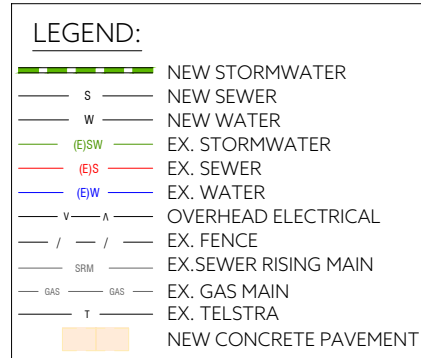
1. ALL AREAS WITHIN COMPLETED STAGES WHERE TOPSOIL HAS BEEN SPREAD SHOULD BE MONITORED FOR SERRATED TUSsock AND TREATED AS REQUIRED.
2. EQUIPMENT MOVEMENT SHOULD BE RESTRICTED TO THE DEVELOPMENT FOOTPRINT. NOTE: TRACKED EQUIPMENT MOVING ACROSS WEED INFESTED COUNCIL LAND HAS HIGH RISK OF PICKING UP WEED SEED AND/OR SPREADING TO COUNCIL LAND.
3. CONTINUE TO ENSURE WASH DOWN LEDGER IS SUBMITTED TO COUNCIL TO DEMONSTRATE WHEN EQUIPMENT IS CLEANED PRIOR TO MOVING OFF SITE.
4. SEDIMENT TRAP SHOULD BE MONITORED FOR EMERGING WEEDS AND SEDIMENT DEEP BURIED AT END OF EACH STAGE PARTICULARLY IF EQUIPMENT HAS BEEN MOVED OFF SITE AND WASH DOWN WILL NOT BE USED IN MID TO LONG TERM.
5. TRUCKS AND EQUIPMENT SHOULD BE CLEAN PRIOR TO COMING TO SITE IF THEY ARE TO LEAVE THE HARDENED SURFACE WHILE ON SITE. ANY VEHICLES/EQUIPMENT THAT LEAVE FORMED ROADS MUST BE CLEANED PRIOR TO LEAVING THE SITE AND A RECORD KEPT IN THE WASH DOWN LEDGER. HYGIENE MEASURES PRIOR TO LEAVING & ENTERING THE SITE TO COMPLY WITH THE TASMANIAN WASHDOWN GUIDELINES FOR WEED & DISEASE CONTROL.
6. THE WASH DOWN LEDGER DOCUMENTING LICENCE PLATE NUMBERS OF ALL VEHICLES, TRUCKS AND EQUIPMENT SHOULD BE SUBMITTED TO COUNCIL TWICE PER YEAR (AS A MINIMUM) TO DEMONSTRATE WHEN EQUIPMENT IS CLEANED/LEAVING THE SITE. WASH DOWN LEDGER MUST BE REGULARLY SUBMITTED TO COUNCIL.
7. CONFINE DISTURBANCE OF TOPSOIL AND EARTHWORK TO A MINIMUM CORRIDOR.
8. STRIP ROAD CORRIDORS FIRST, THEN CONFINE MOVEMENTS OF WORKS VEHICLES TO THE FUTURE ROAD CORRIDORS, WHERE POSSIBLE.
9. ESTABLISH A STABILISED ENTRANCE AND WASHDOWN AREA JUST INSIDE THE WORKS BOUNDARY, IN ACCORDANCE WITH DRAWING.
10. ALL VEHICLES LEAVING THE SITE TO BE CLEANED AND WASHED DOWN PRIOR TO EXIT.
11. PRIVATE VEHICLES SHOULD NOT BE PARKED WITHIN THE SITE.
12. STOCKPILE AREAS TO BE PLACED CLOSE TO THE SITE ENTRANCE WHERE POSSIBLE.
13. WARNING SIGNAGE IS TO BE PLACED AT THE VEHICLE WASHDOWN BAY STATING THE NEED FOR ALL VEHICLES TO WASHDOWN PRIOR TO LEAVING THE SITE.
14. ANY IMPORTED FILL MATERIALS MUST BE SOURCED FROM QUARRIES ABLE TO PROVIDE DOCUMENTATION AS TO THE WEEDS PRESENT ON THE SOURCE SITE IN ORDER TO MINIMISE THE INTRODUCTION OF NEW WEEDS AND PATHOGENS TO THE AREA.

1. REFER TO A53500.1 FOR PRIVATE WATER PLUMBING
2. REFER TO A53500.2 FOR PRIVATE SEWER PLUMBING
3. REFER TO A53500.3 FOR PRIVATE STORMWATER DRAINAGE PLUMBING

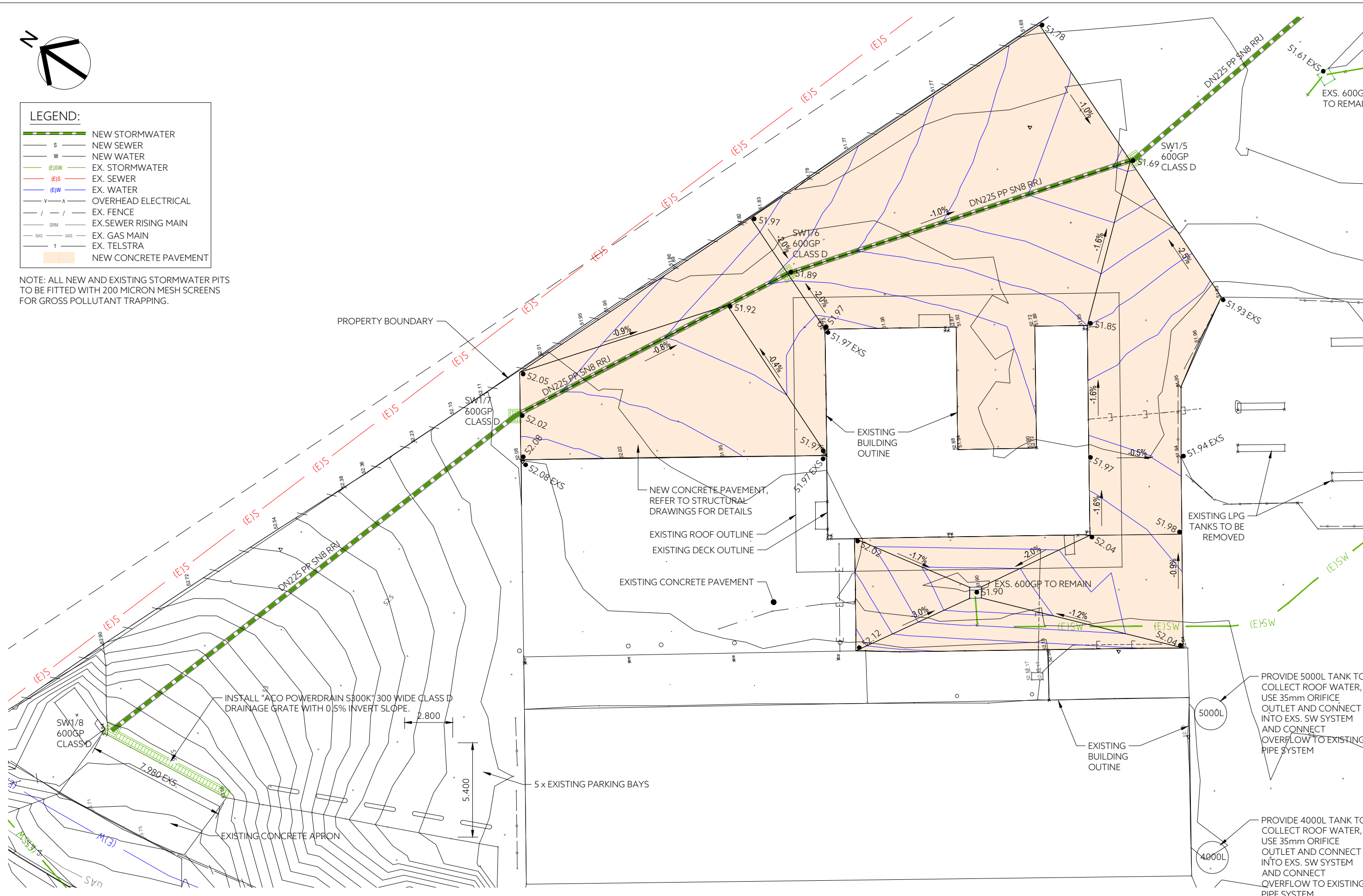
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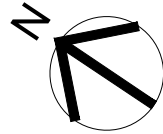


NOTE: ALL NEW AND EXISTING STORMWATER PITS
TO BE FITTED WITH 200 MICRON MESH SCREENS
FOR GROSS POLLUTANT TRAPPING.



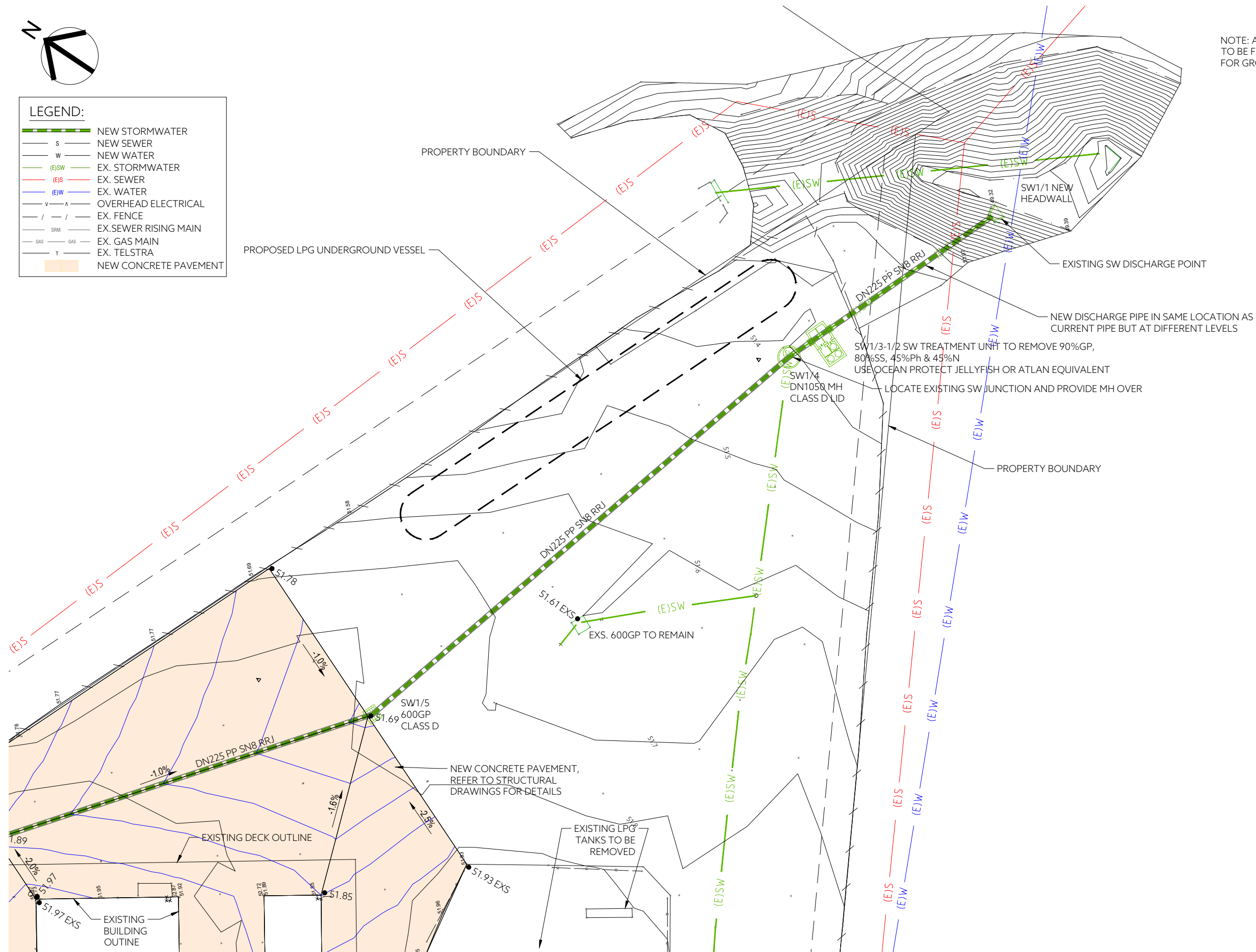
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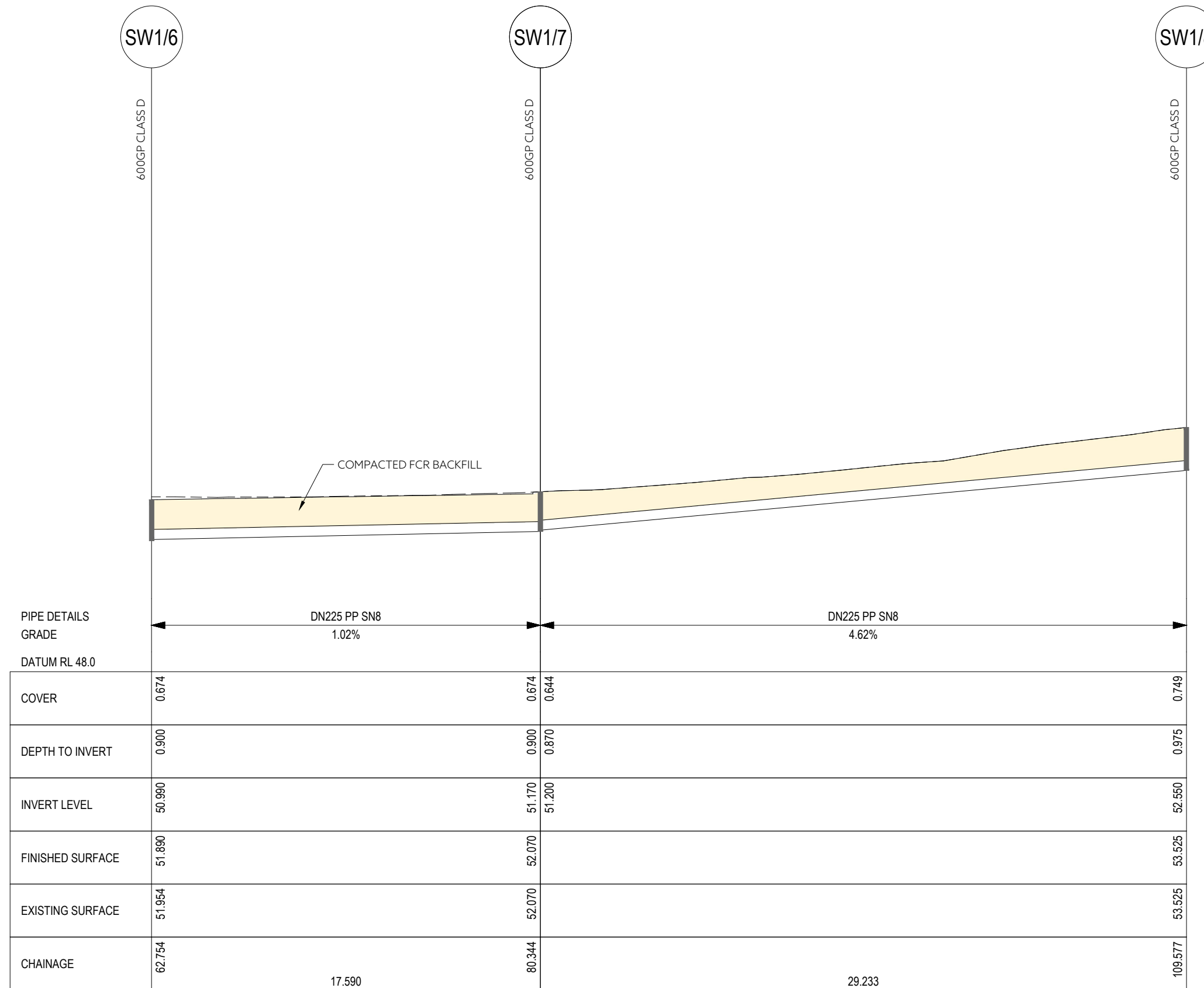
LEGEND:	
	NEW STORMWATER
	NEW SEWER
	NEW WATER
	EX. STORMWATER
	EX. SEWER
	EX. WATER
	OVERHEAD ELECTRICAL
	EX. FENCE
	EX. SEWER RISING MAIN
	EX. GAS MAIN
	EX. TELSTRA
	NEW CONCRETE PAVEMENT

NOTE: ALL NEW AND EXISTING STORMWATER PITS TO BE FITTED WITH 200 MICRON MESH SCREENS FOR GROSS POLLUTANT TRAPPING.



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NOTE: REFER TO AS3500.3 FOR PRIVATE STORMWATER REGULATIONS



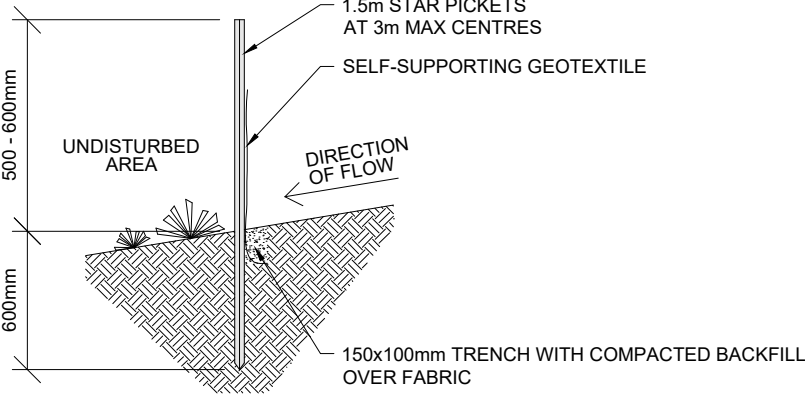
LONG SECTION - LINE SW1

SCALE: HORIZ 1:200 VERT 1:100

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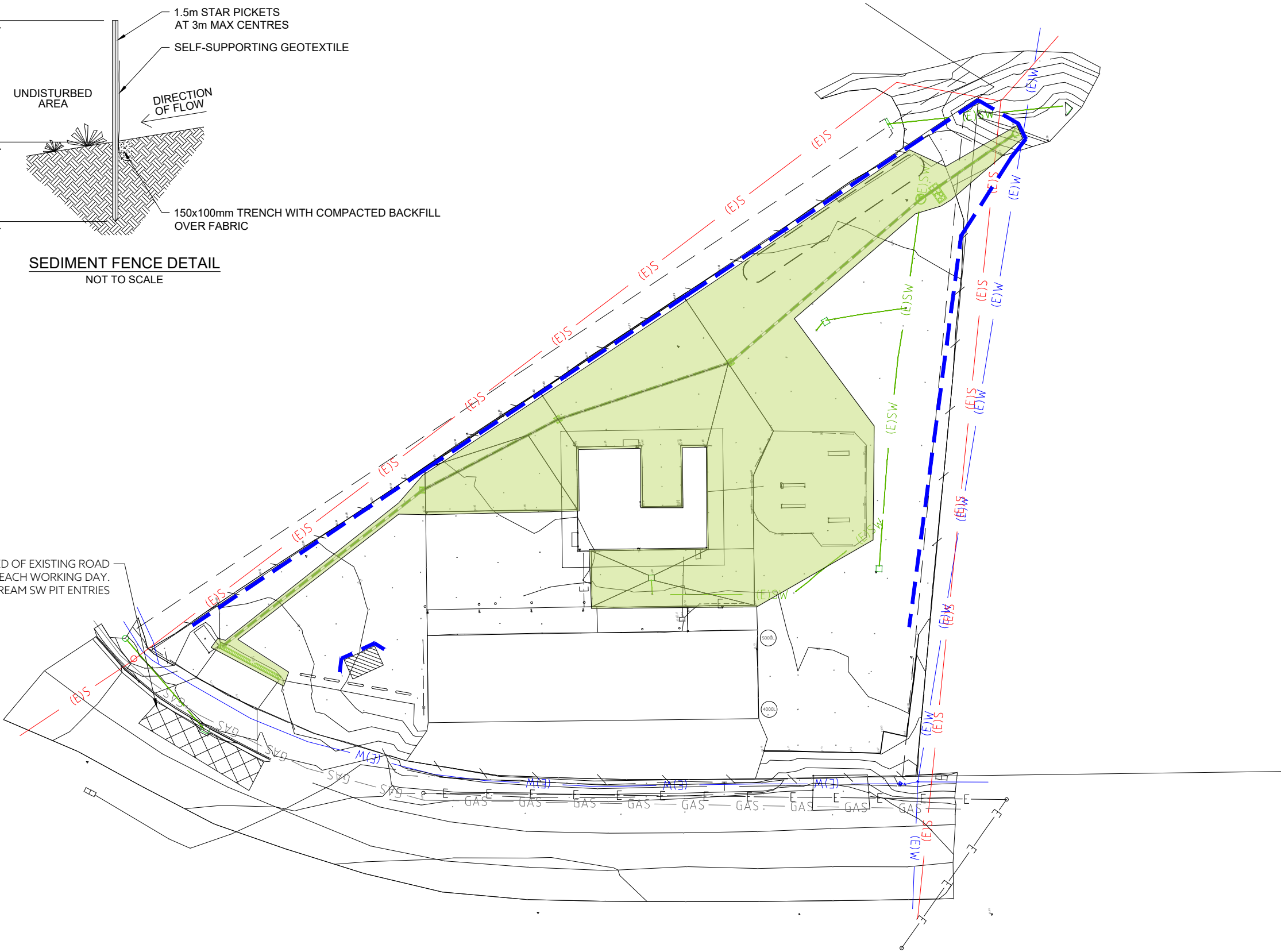
LEGEND:

- AREAS OF GROUND DISTURBANCE
- SEDIMENT FENCE
- PROPOSED STOCKPILE AREA

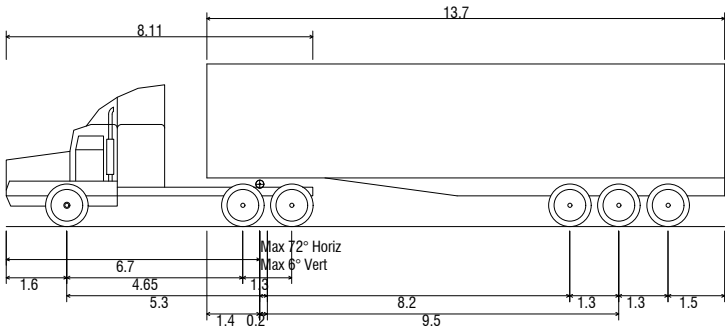
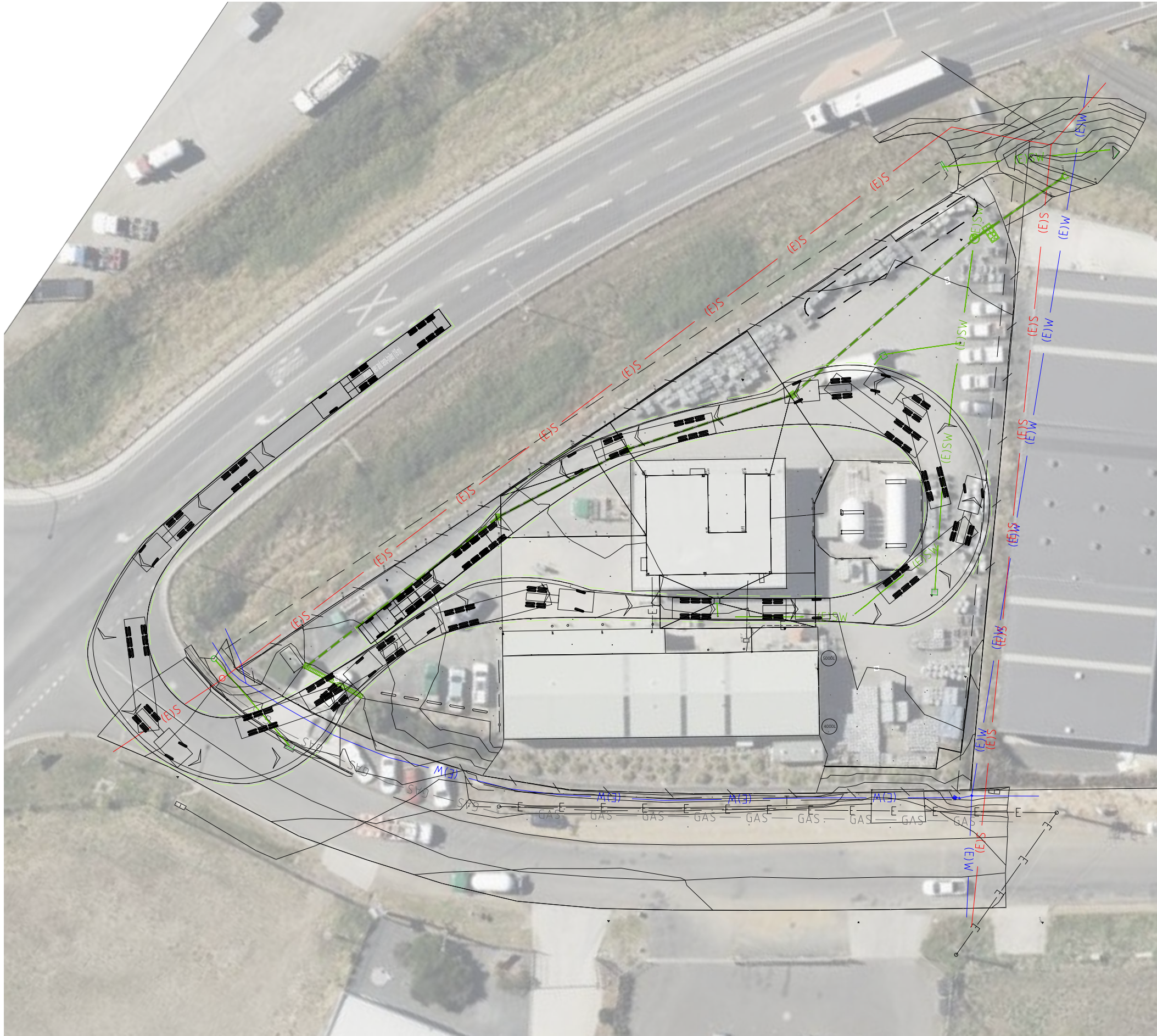


SEDIMENT FENCE DETAIL
NOT TO SCALE

ENSURE MUD AND DIRT IS CLEANED OF EXISTING ROAD
AT END OF EACH WORKING DAY.
PLACE FILTER SOCKS AT DOWNSTREAM SW PIT ENTRIES



REV	AMENDMENTS	DRAWN	DATE	APPR.	THIS SHEET MAY BE PRINTED USING COLOUR AND MAY BE INCOMPLETE IF COPIED	ISSUED DATE: 27/06/2025	JOB MANAGER: MARK WESTERBERG	CLIENT: DDM CIVIL - SUPAGAS PROJECT DESCRIPTION: PAVEMENT LEVELS AND DRAINAGE ADDRESS: 28 CROOKED BILLET DRIVE, BRIGHTON DRAWING TITLE: SOIL AND WATER MANAGEMENT PLAN	DESIGNED: SC DRAWN: SC JOB MANAGER: MARK WESTERBERG ISSUED DATE: 27/06/2025	REVIEWED: MW REVIEWED: MW	CONTRACT NO. ----- JOB NUMBER -----	SCALE 1: 500 DISCIPLINE C	PAPER (A3) SHEET 900	REVISION P2
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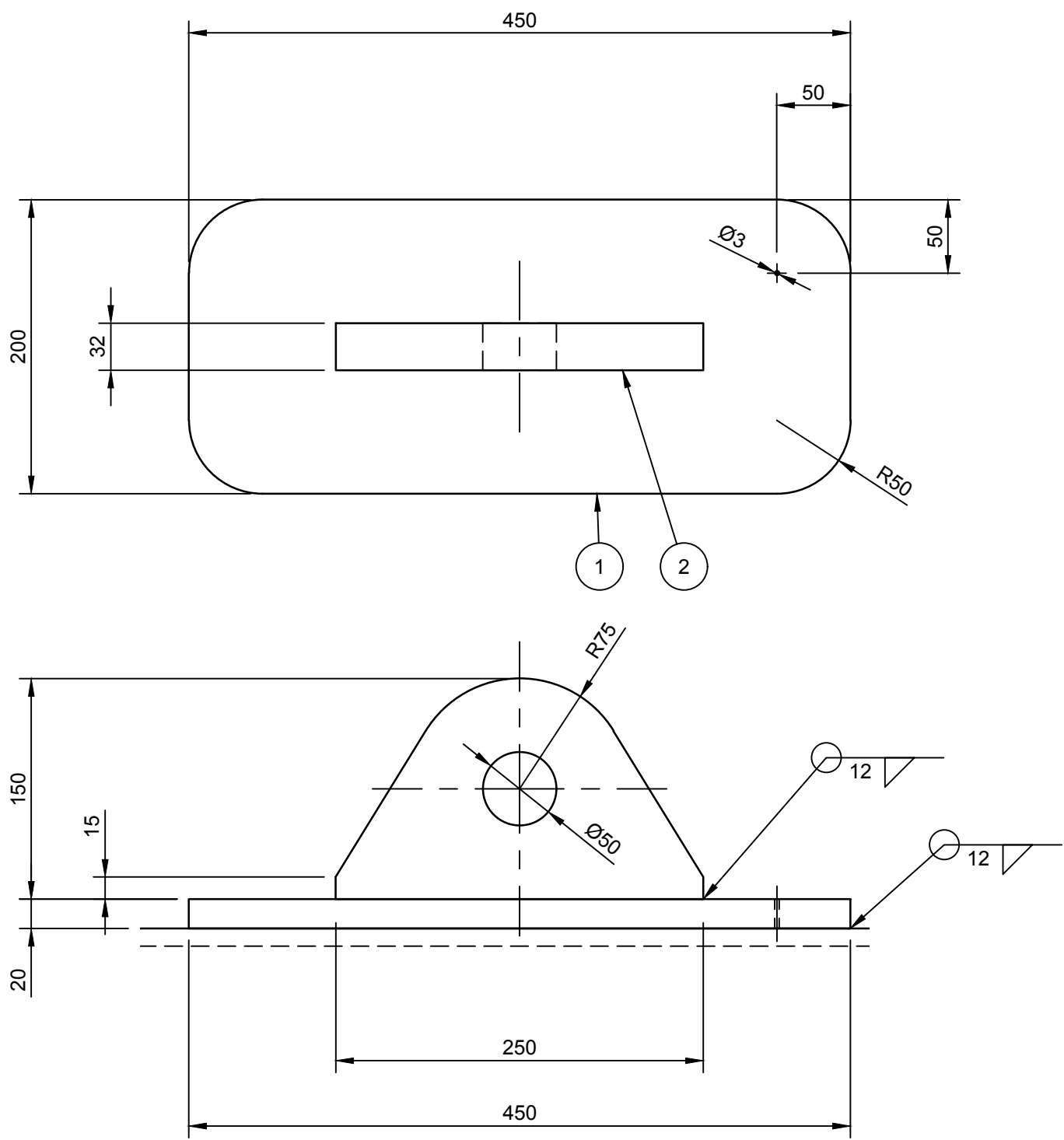


Prime mover and semi-trailer (19 m)	
Overall Length	19.000m
Overall Width	2.500m
Overall Body Height	4.300m
Min Body Ground Clearance	0.540m
Track Width	2.500m
Lock-to-lock time	6.00s
Curb to Curb Turning Radius	12.500m

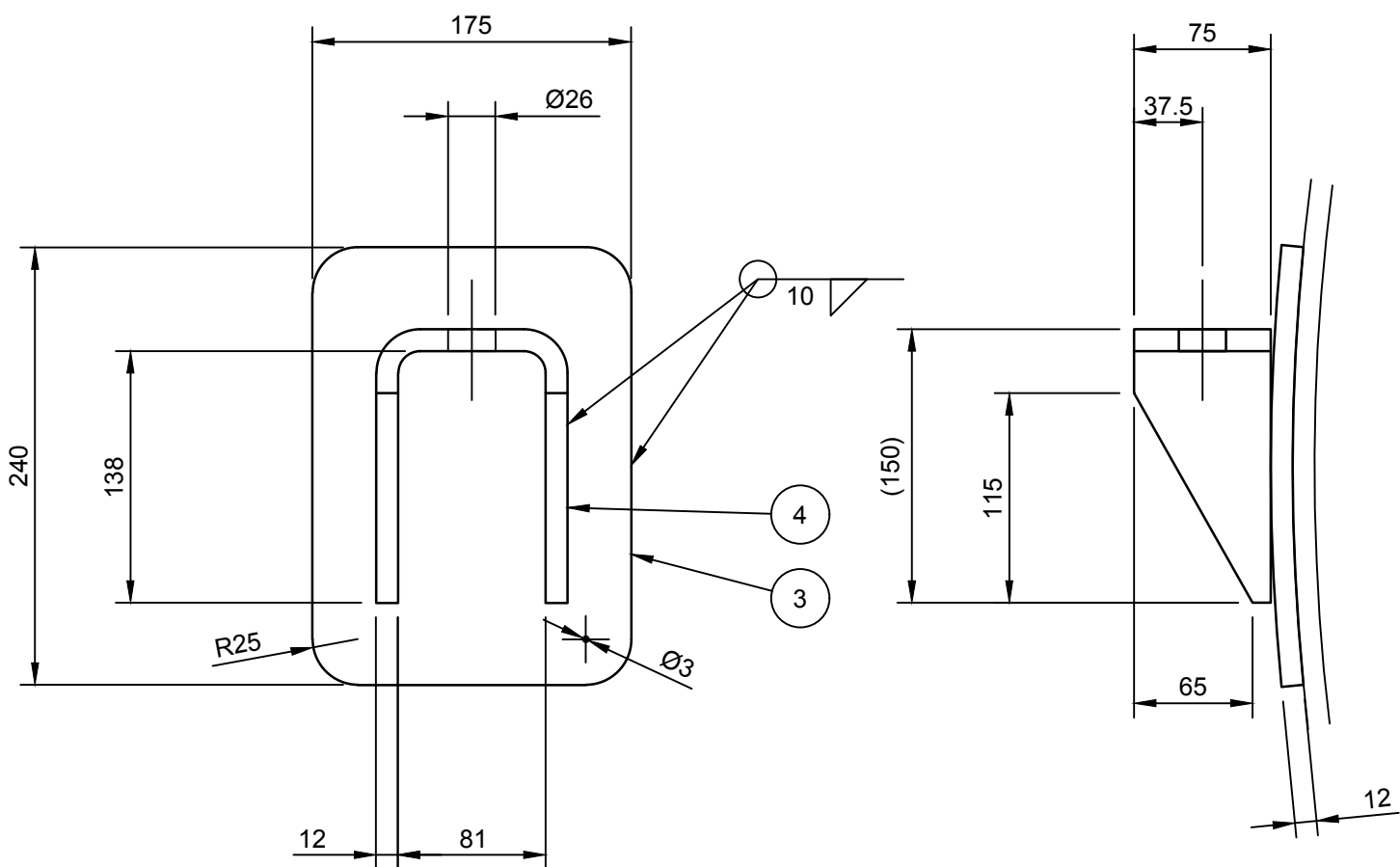
DESIGN VEHICLE PROFILE

DRAWING STATUS:				DESIGNED:	REVIEWED:	CLIENT:	DDM CIVIL - SUPAGAS PAVEMENT LEVELS AND DRAINAGE 28 CROOKED BILLET DRIVE, BRIGHTON VEHICLE TURNING PATHS
FOR PLANNING APPROVAL				SC	MW	PROJECT DESCRIPTION:	
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PLANAR				SC	MW	DRAWING TITLE:	
THIS SHEET MAY BE PRINTED USING COLOUR AND MAY BE INCOMPLETE IF COPIED				JOB MANAGER: MARK WESTERBERG			
REV	AMENDMENTS	DRAWN	DATE	APPR.	ISSUED DATE:	27/06/2025	
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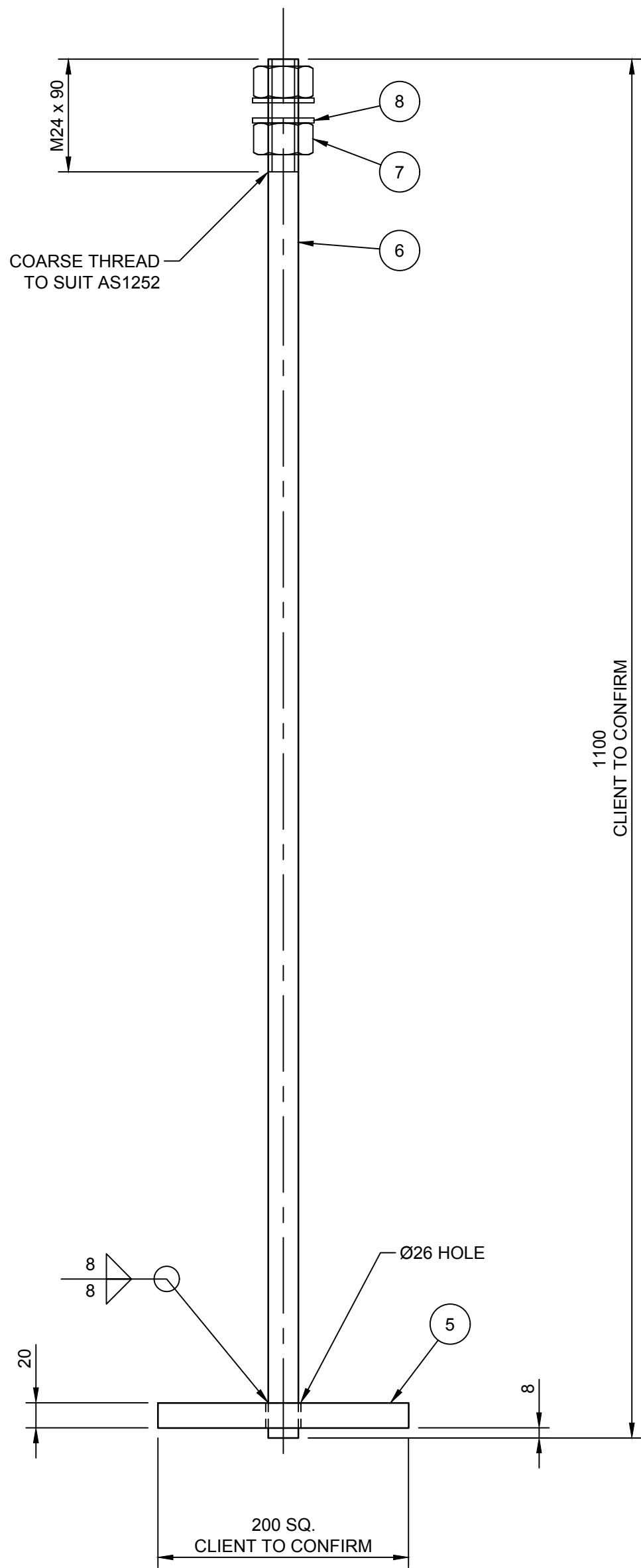
 PDA SURVEYORS, ENGINEERS & PLANNERS	127 Bathurst Street Hobart, Tasmania, 7000 PHONE: +61 03 6234 3217 EMAIL: pda.hbt@pda.com.au www.pda.com.au Also at: Kingston, Launceston & Burnie		CONTRACT NO.	SCALE	PAPER
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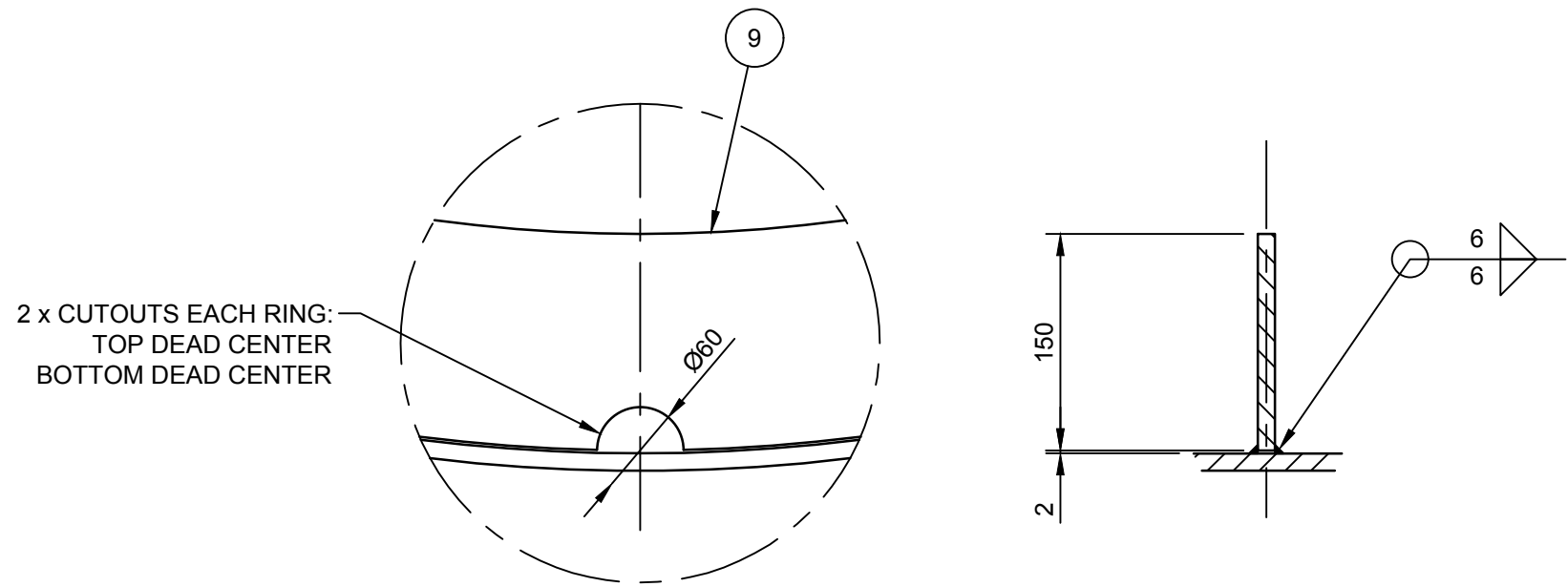
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2 REQ'D
SCALE 1:4



H.D. LUG
16 REQ'D
SCALE 1:4



H.D. ROD / ANCHOR
16 REQ'D
SCALE 1:4



TYP STIFFENER RING
SCALE 1:5

9	STIFF. RING	4	12 PL, 2434 O.D. x 2134 I.D.	AS1548-PT460NR	
8	WASHER	32	M24 FLAT WASHER	AS1252	ZINC. PL.
7	NUT	32	M24 HEX NUT Gr 8.8	AS1252	ZINC. PL.
6	ROD	16	Ø24 RD BAR	AS3679.1-300	
5	ANCHOR PLATE	16	20 PL, 200 x 200	AS3678-250	
4	H.D. LUG	16	12 PL, 366 x 75 (FLAT PATTERN)	AS1548-PT460NR	
3	REINF. PAD	16	12 PL, 240 x 175	AS1548-PT460NR	ROLL TO VESSEL O.D.
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1	REINF. PAD	2	20 PL, 450 x 200	AS1548-PT460NR or AS3678-250	ROLL TO VESSEL O.D.
ITEM	DESCRIPTION	QTY	DIMENSIONS	MATERIAL	COMMENTS

L & A

WELDING

L & A PRESSURE WELDING PTY LTD

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ACN 002 241 590

LIFTING LUG, STIFFENER RING & H.D. DETAILS

SUPAGAS

LPG UNDERGROUND VESSEL

T-001

A1

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T. O'DEMPSEY

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Q. GRYL

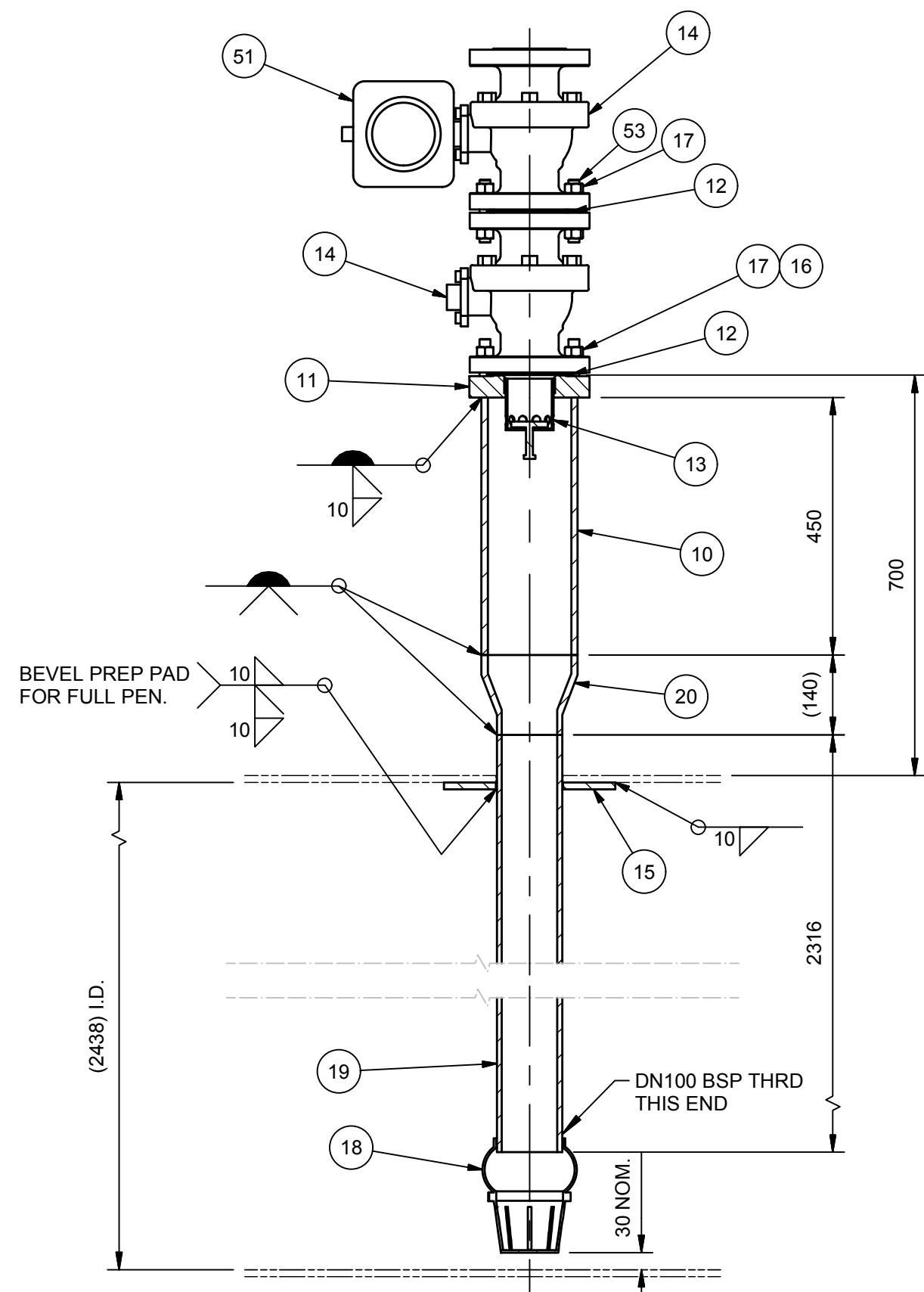
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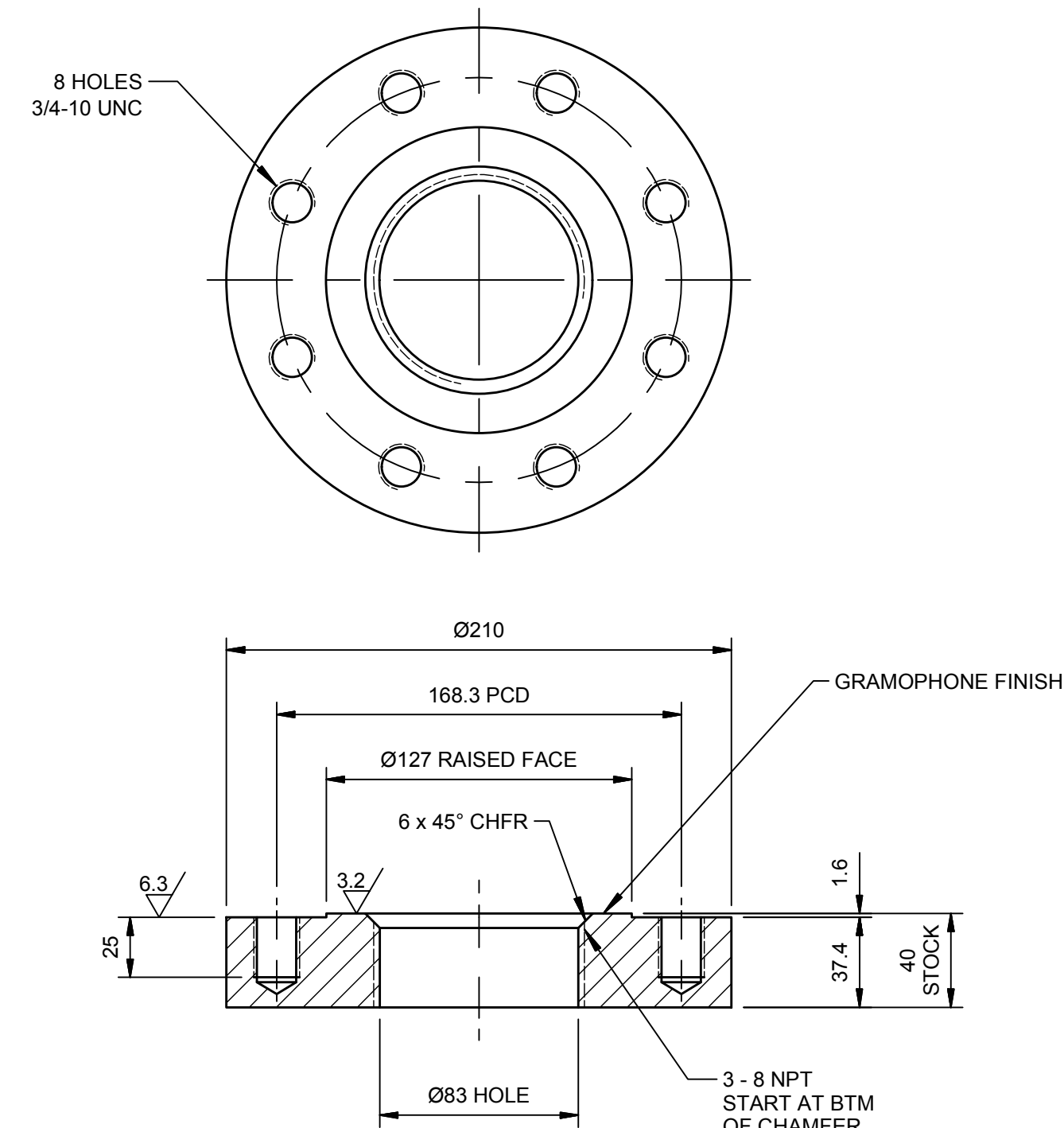
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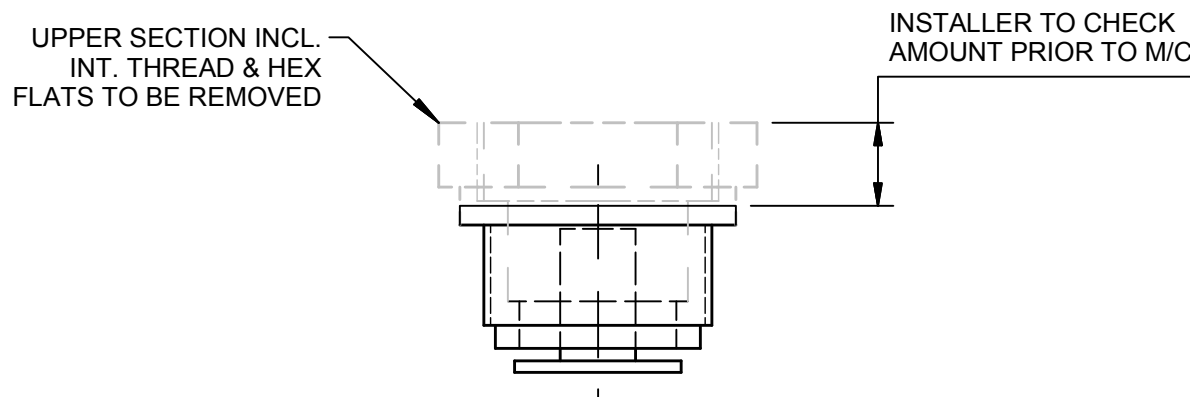
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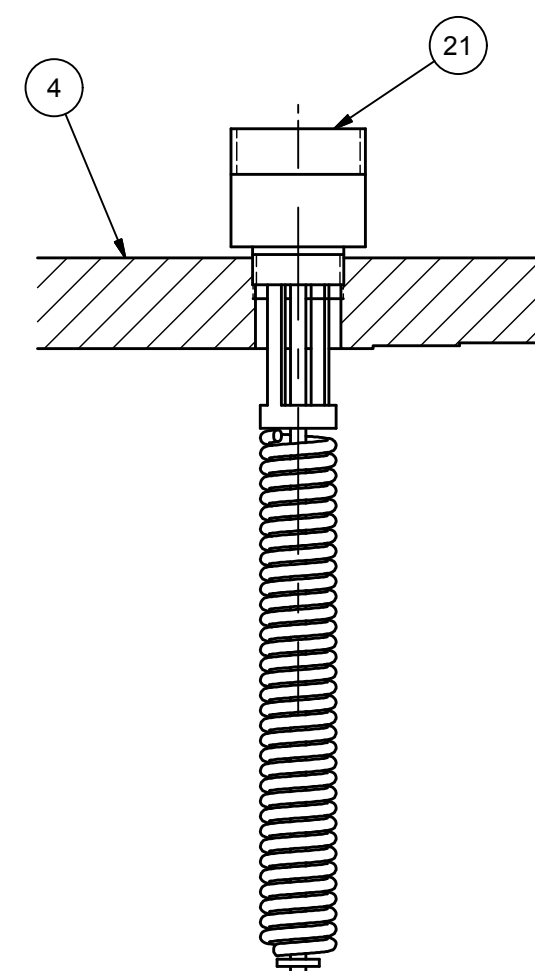
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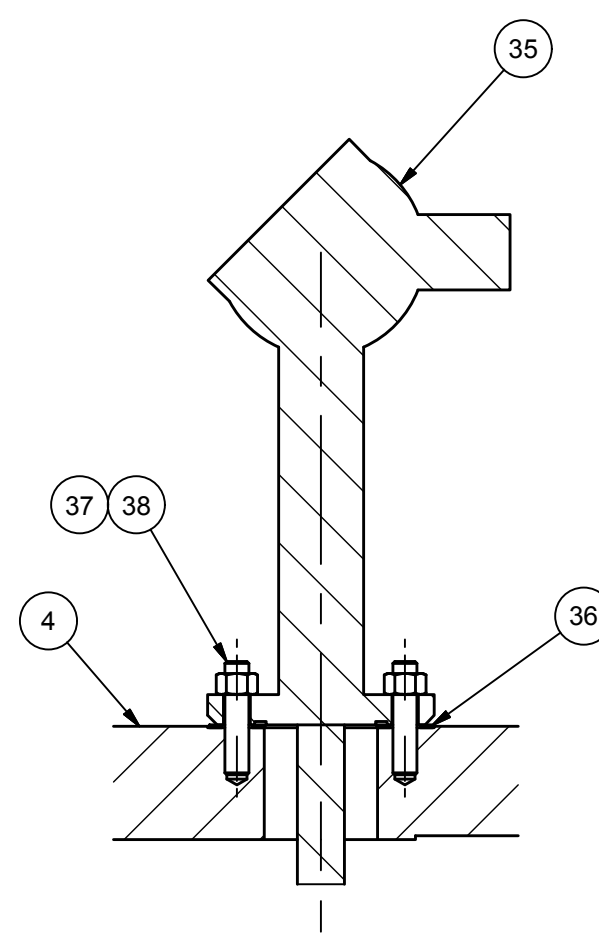
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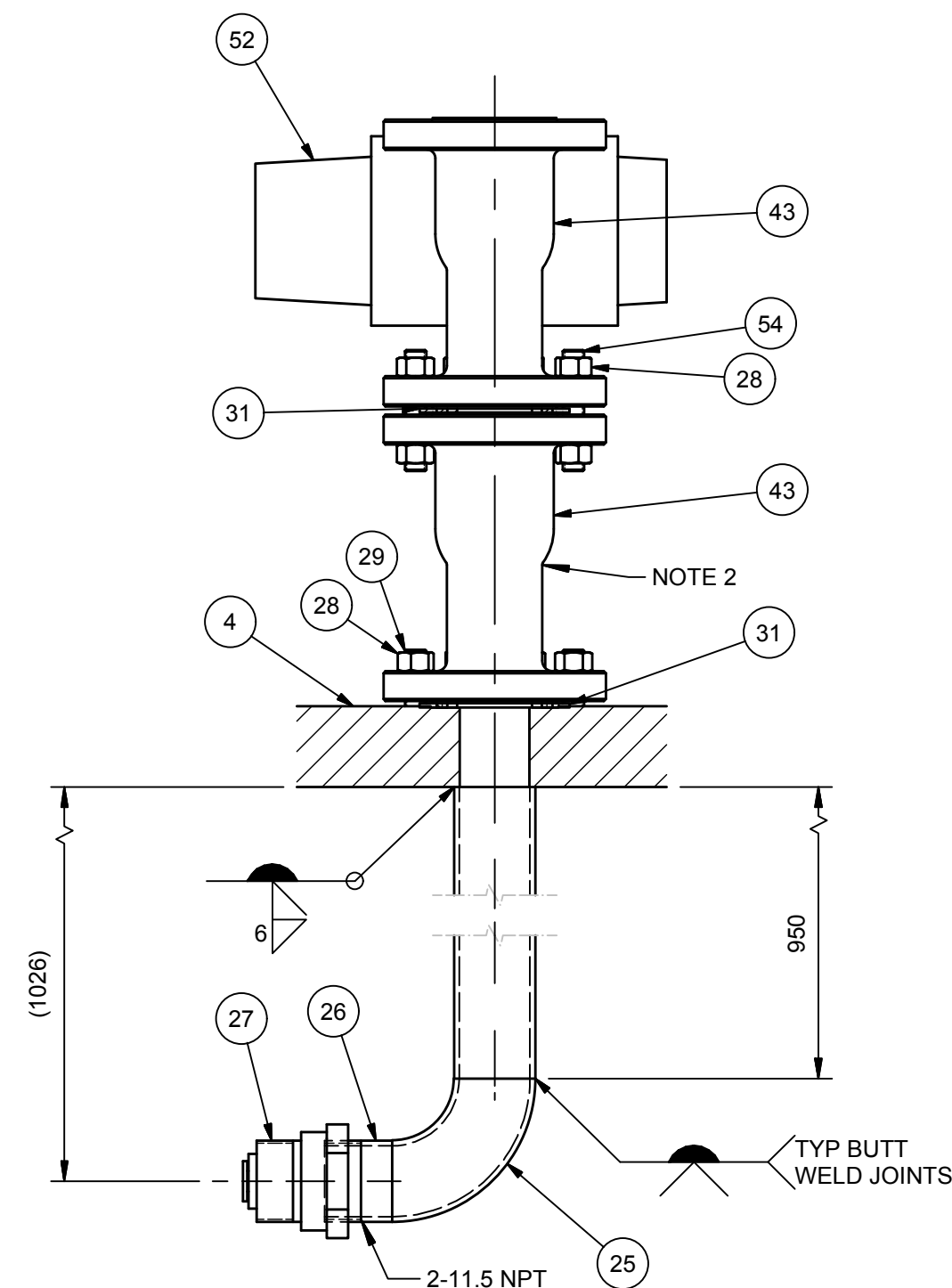
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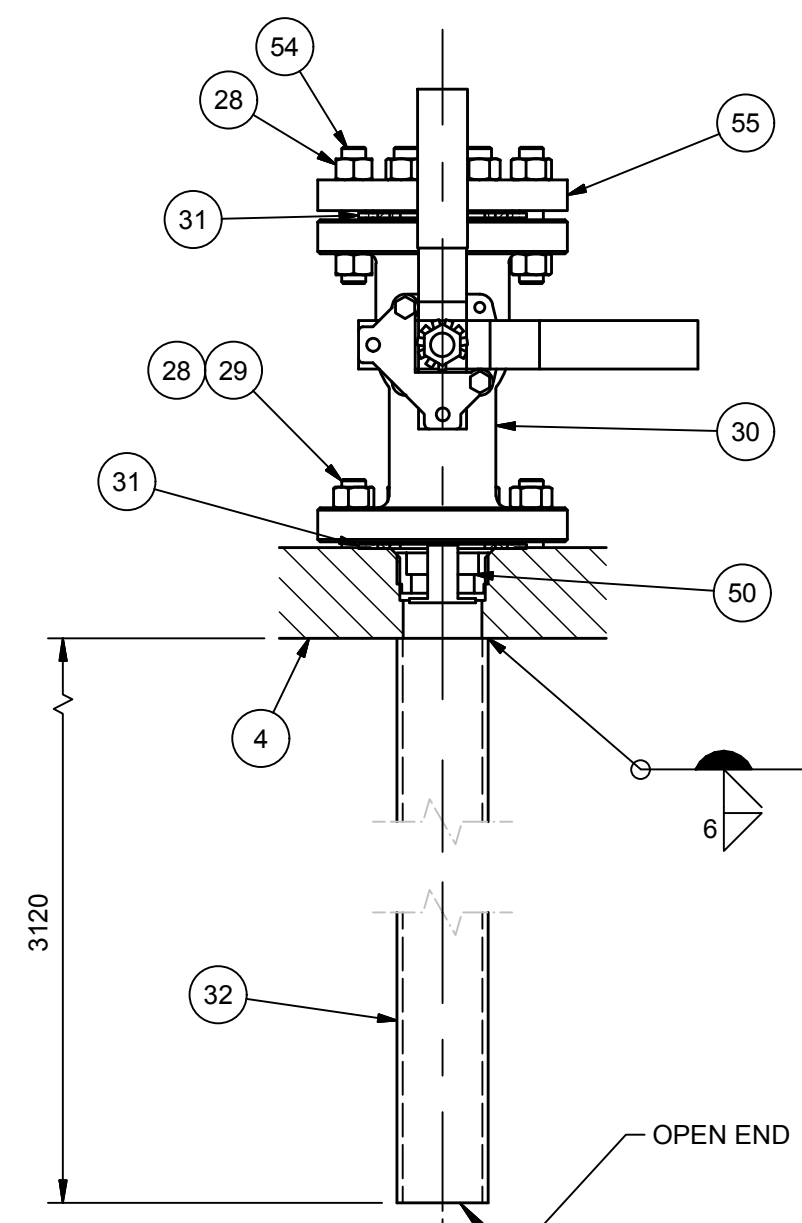
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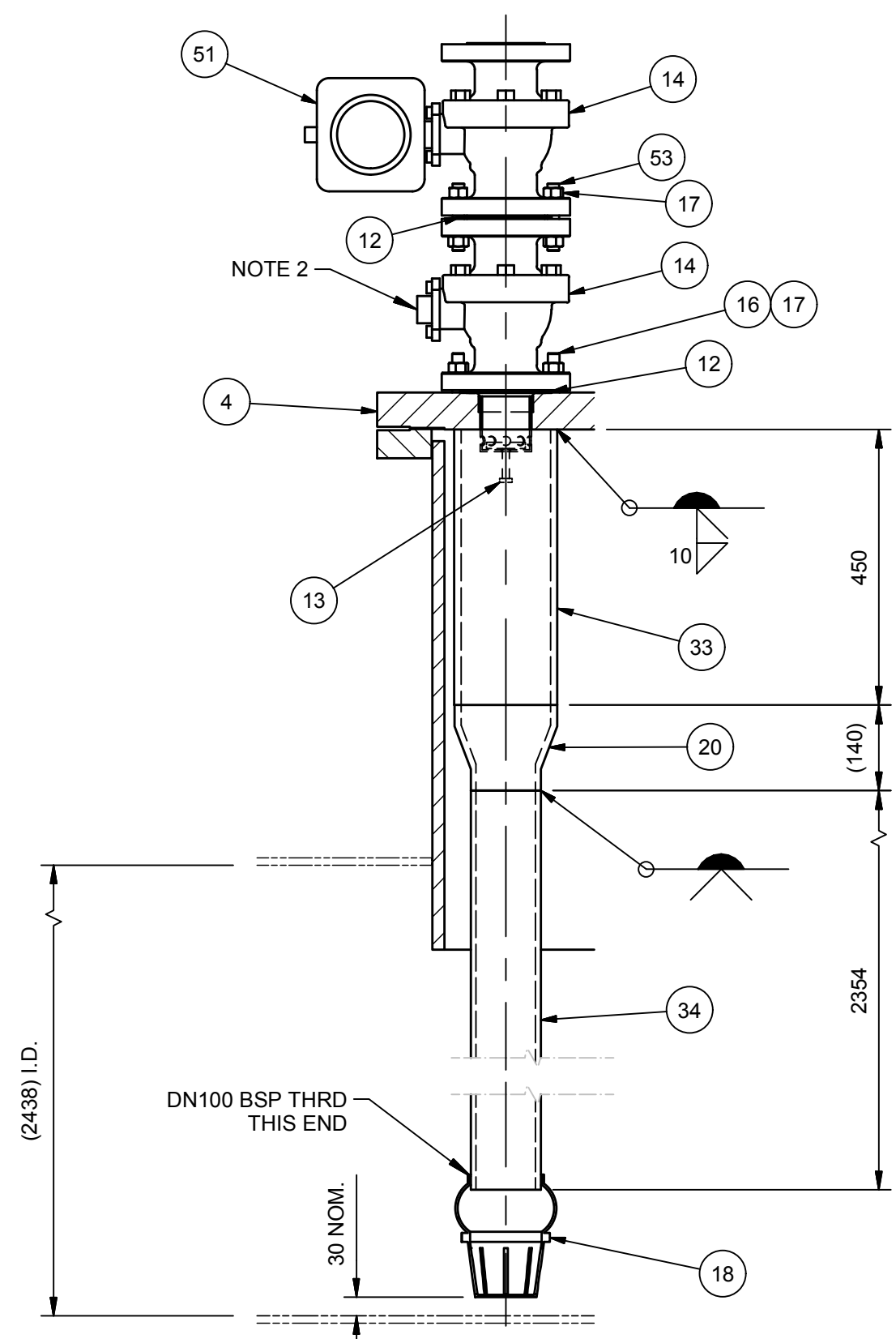
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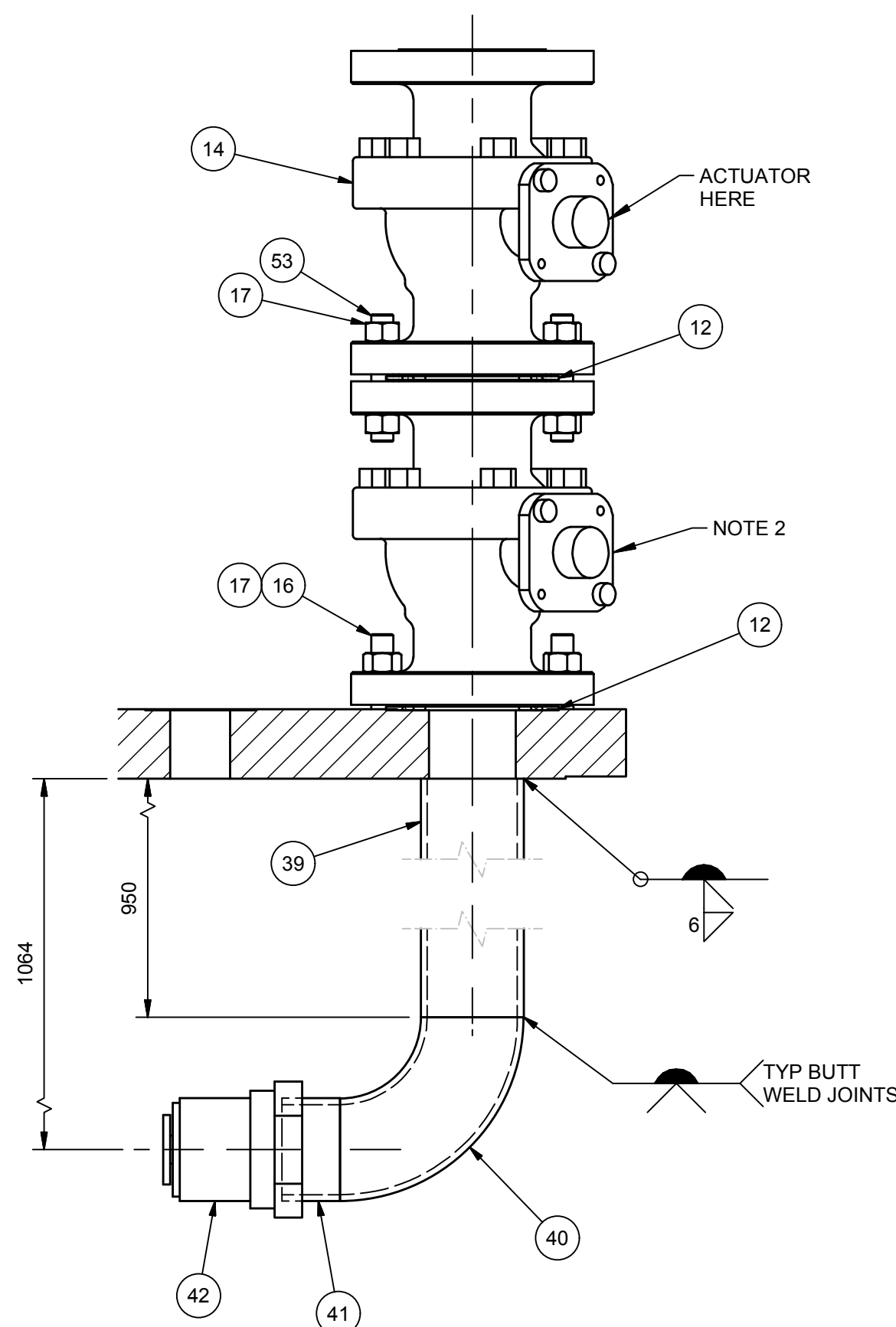
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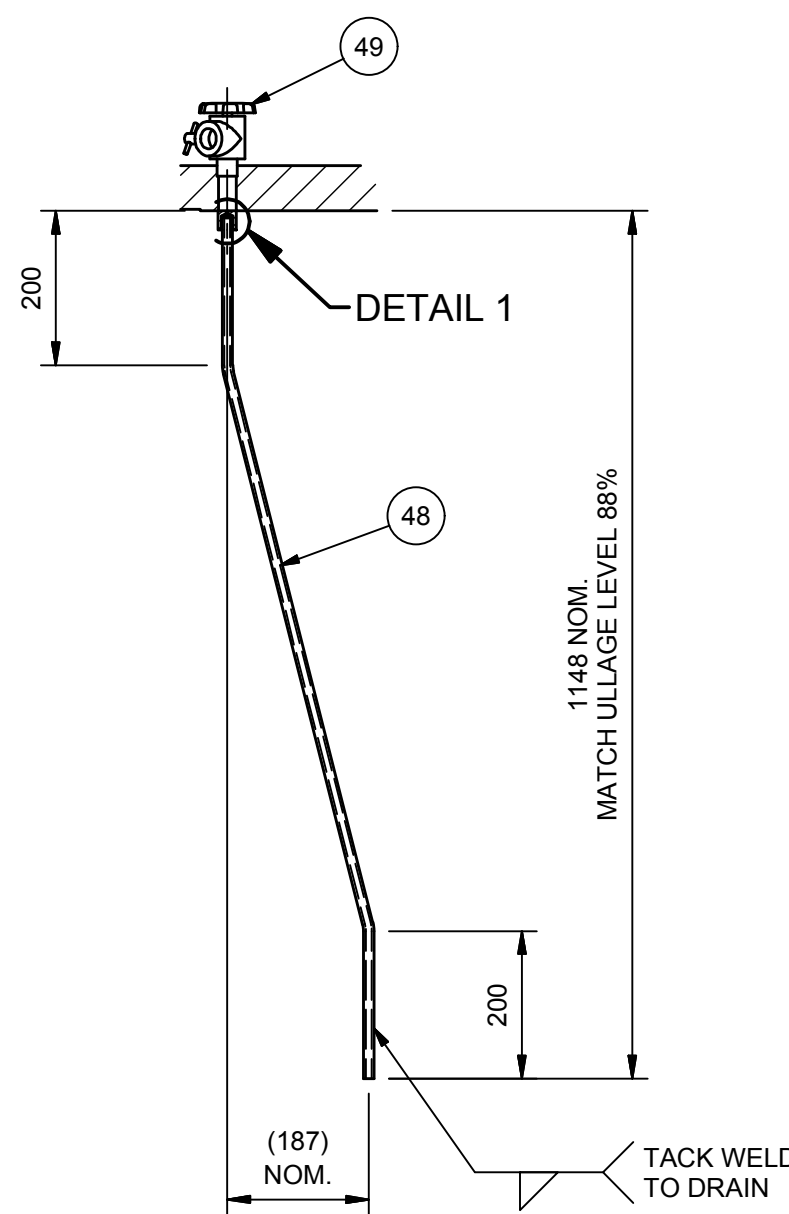
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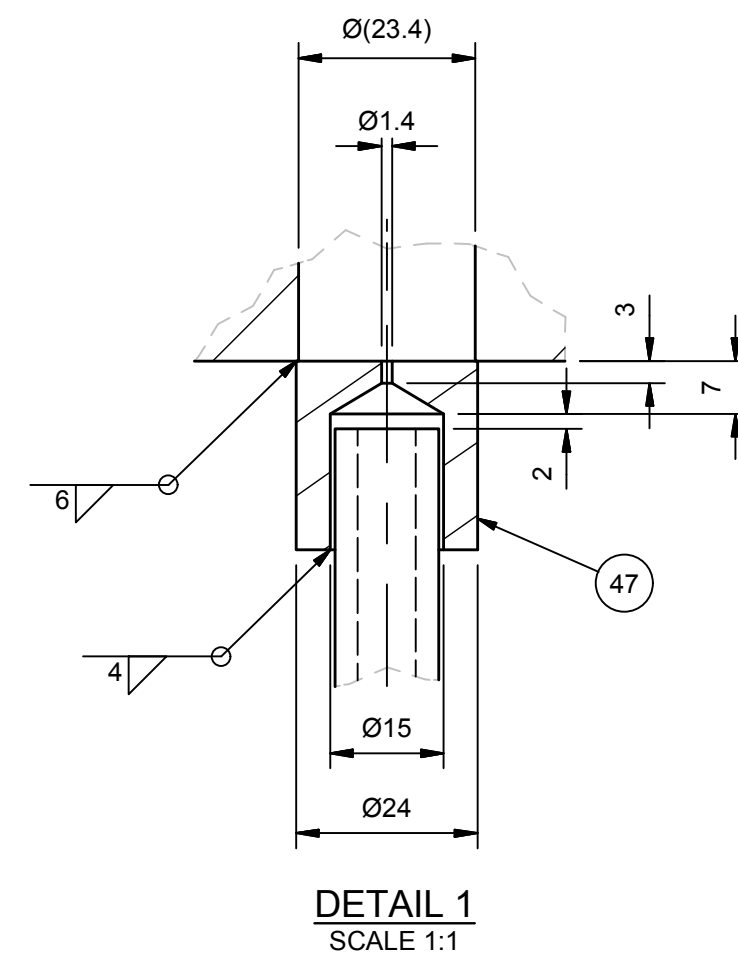
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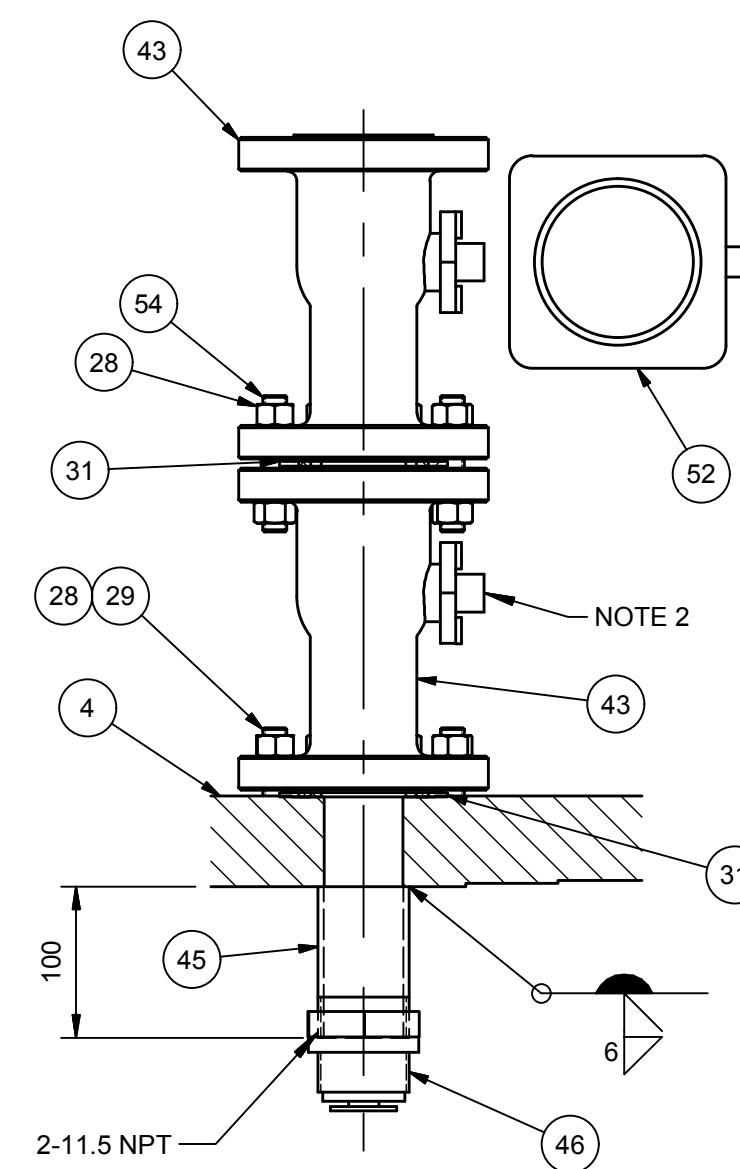
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NOZZLE I
FIXED ULLAGE
SCALE 1:10



DETAIL 1
SCALE 1:1



NOZZLE H
VAPOUR
SCALE 1:5

NOTES:
1. FOR ORIENTATION OF PIPEWORK, B.O.M. ITEMS NOT LISTED ON THIS DRG AND TURRET COVER MACHINING DETAILS, REFER TO DRG LAP-3562-03
2. HANDLES FOR MANUAL VALVES AT NOZZLES C, E, G & H NOT SHOWN THIS DRG BUT SHALL BE INSTALLED WITH HANDLES CUT SHORT

2	REVISED PER MARK-UPS	TOD	QG	QG	15/2/2017
1	DETAIL FOR NOZZLE F CONFIRMED	TOD	QG	QG	8/2/2017
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REV	DETAILS	DRN	CHK	APP	DATE

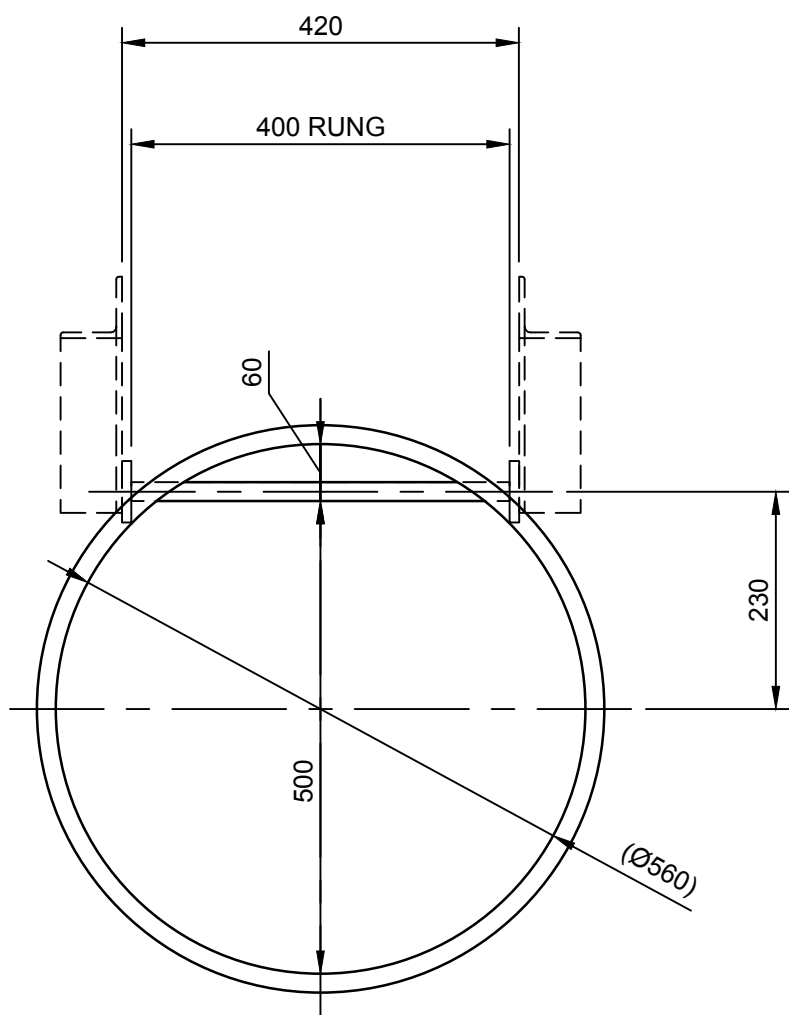
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53	24	STUD BOLT	3/4" UNC x 110 LG		ASTM A193 Gr B7
52	2	PNEUM. ACTUATOR	TO SUIT DN50 MARS BALL VALVE		
51	3	PNEUM. ACTUATOR	TO SUIT DN80 MARS BALL VALVE		
50	1	EXCESS FLOW VALVE, MODIFIED	REGO A3292C, 2" NPT (SEE DETAIL)		BRASS
49	1	ANGLE VALVE	REGO A2805C		
48	1	ULLAGE PIPE	DN8 SCH80 SMLS PIPE ASME B16.10	1152	ASTM A106-B
47	1	ULLAGE SOCKET	Ø24 RD. BAR (SEE DETAIL)	25	AS/NZS 3679.1-300
46	1	EXCESS FLOW VALVE	REGO A3292C, 2" NPT		CADMIUM / SS
45	1	PIPE	DN50 SCH 40, SMLS ASME B36.10M	100	ASTM A106 Gr.B
43	4	BALL VALVE	DN50 CL300 RF FLG, 2 PC, FULL PORT		MARS SERIES 90D
42	1	CHECK VALVE	REGO A3196, 3" NPT		CADMIUM / SS
41	1	PIPE	DN80 SCH 40, SMLS ASME B36.10M	50	ASTM A106 Gr.B
40	1	ELBOW, LR	DN80 SCH40, 90°, ASME B16.9		ASTM A234-WPB
39	1	PIPE	DN80 SCH 40, SMLS ASME B36.10M	950	ASTM A106 Gr.B
38	8	STUD BOLT	1/2" UNC x 60 LG		ASTM A193 Gr B7
37	8	HEX NUT	1/2" UNC		ASTM A194 Gr 2H
36	1	GASKET	TO SUIT BINTECH B12000 FLANGE		
35	1	LEVEL SENSOR	BINTECH B12000		
34	1	PIPE	DN100 SCH 80, SMLS ASME B36.10M	2354	ASTM A106 Gr.B
33	1	PIPE	DN150 SCH 80, SMLS ASME B36.10M	450	ASTM A106 Gr.B
32	1	PIPE	DN50 SCH 40, SMLS ASME B36.10M	3120	ASTM A106 Gr.B
31	6	GASKET	DN50 SPIRAL WD, CL300, 316SS RINGS		ASME B16.20
30	1	BALL VALVE, MANUAL	DN50 CL300 RF FLG, 2 PC, FULL PORT		MARS SERIES 90D
29	24	STUD BOLT	5/8" UNC x 70 LG		ASTM A193 Gr B7
28	72	HEX NUT	5/8" UNC		ASTM A194 Gr 2H
27	1	CHECK VALVE	REGO A3186, 2" NPT		CADMIUM / SS
26	1	PIPE	DN50 SCH 40, SMLS ASME B36.10M	50	ASTM A106 Gr.B
25	1	ELBOW, LR	DN50 SCH40, 90°, ASME B16.9		ASTM A234-WPB
24	1	PIPE	DN50 SCH 40, SMLS ASME B36.10M	950	ASTM A106 Gr.B
21	1	PSV	FISHER H284 - 225 psi 2" NPT		BRASS / SS
20	2	REDUCER, CONC	DN150 x DN100 SCH80, ASME B16.9		ASTM A234-WPB
19	1	PIPE	DN100 SCH 80, SMLS ASME B36.10M	2316	ASTM A106 Gr.B
18	2	FOOT VALVE	DN100 BAKER'S VBF 94, BSP THRD		BRASS
17	72	HEX NUT	3/4" UNC		ASTM A194 Gr 2H
16	24	STUD BOLT	3/4" UNC x 90 LG		ASTM A193 Gr B7
15	1	REINF. PAD	PLATE, 12 x 300 O.D. x 117 I.D.		AS1548-PT460NR
14	6	BALL VALVE	DN80 CL300 RF FLG, 2 PC, FULL PORT		MARS SERIES 90D
13	2	EXCESS FLOW VALVE	REGO A3500V6		CADMIUM / SS
12	6	GASKET	DN80 SPIRAL WD, CL300, 316SS RINGS		ASME B16.20
11	1	FLANGE, NOZZLE A	40 PL, Ø210		AS1548-PT460NR
10	1	PIPE	DN150 SCH 80, SMLS ASME B36.10M	450	ASTM A106 Gr.B
ITEM	QTY	DESCRIPTION	STOCK	LG	SPEC

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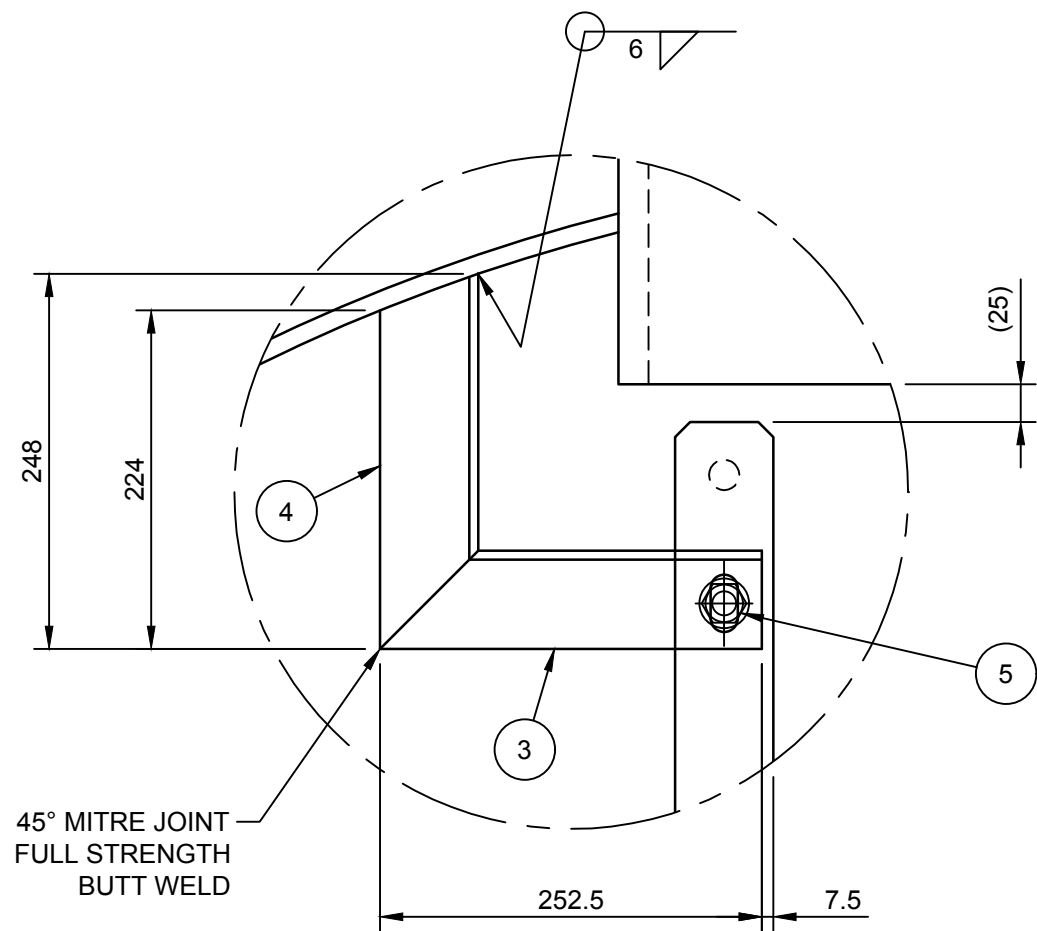
TURRET & NOZZLE DETAILS, SHT 2
SUPAGAS
LPG UNDERGROUND VESSEL
T-001

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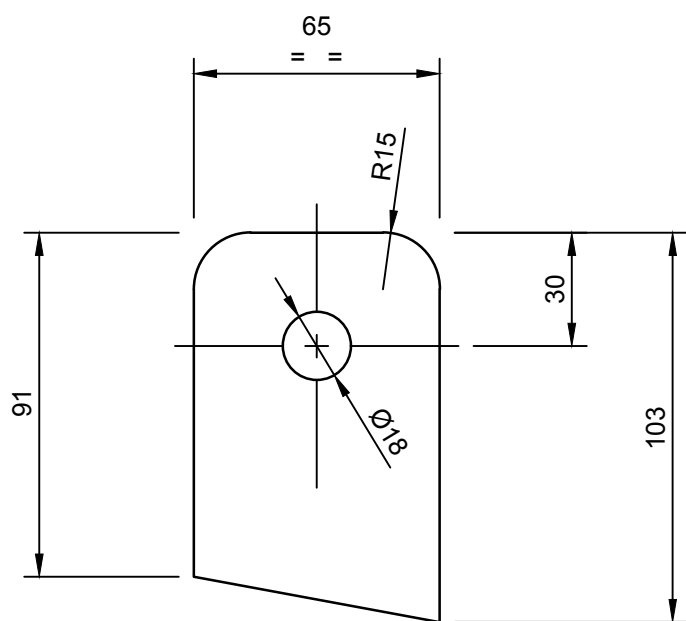
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 - REMOVE ALL SHARP EDGES AND WELD SPLATTER



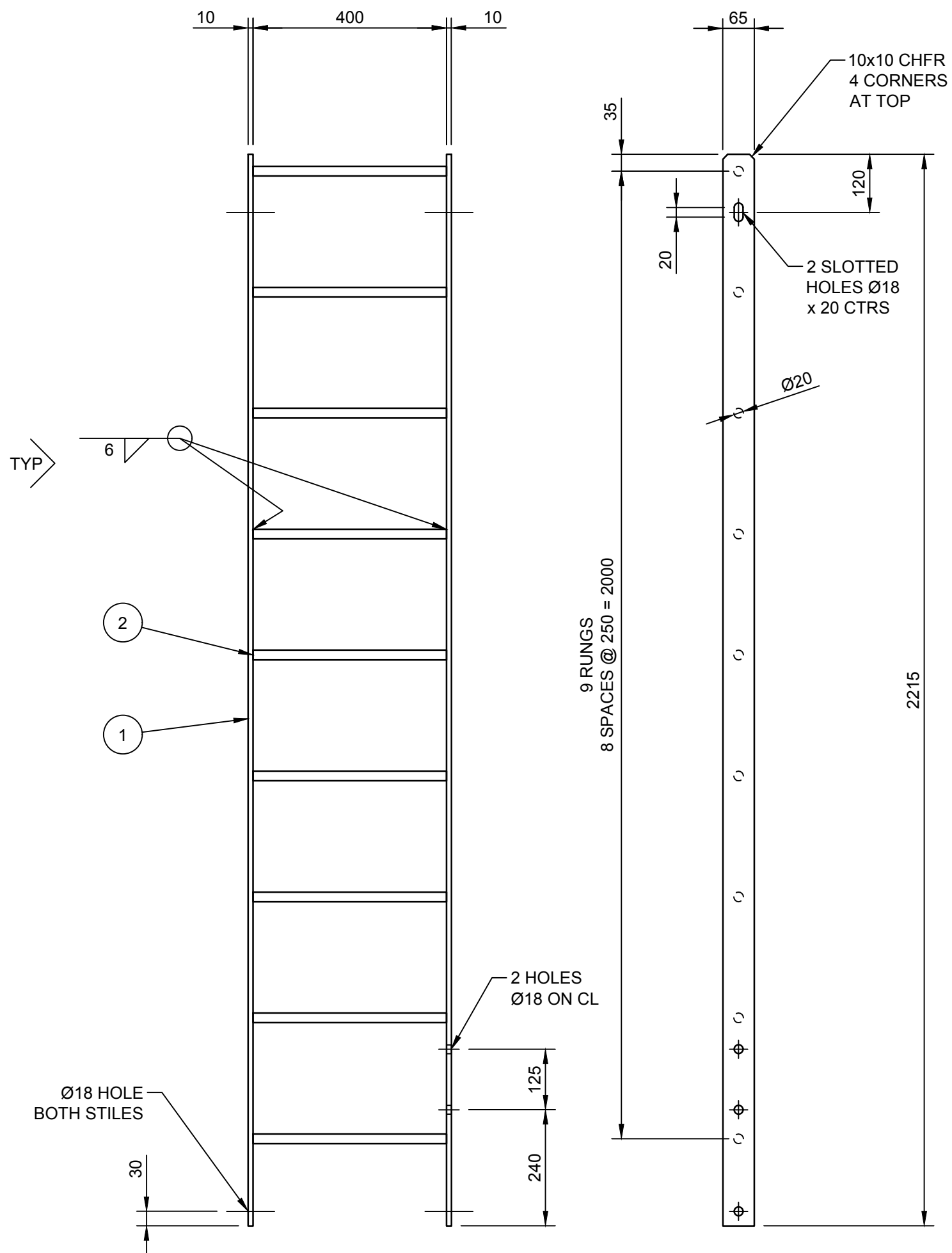
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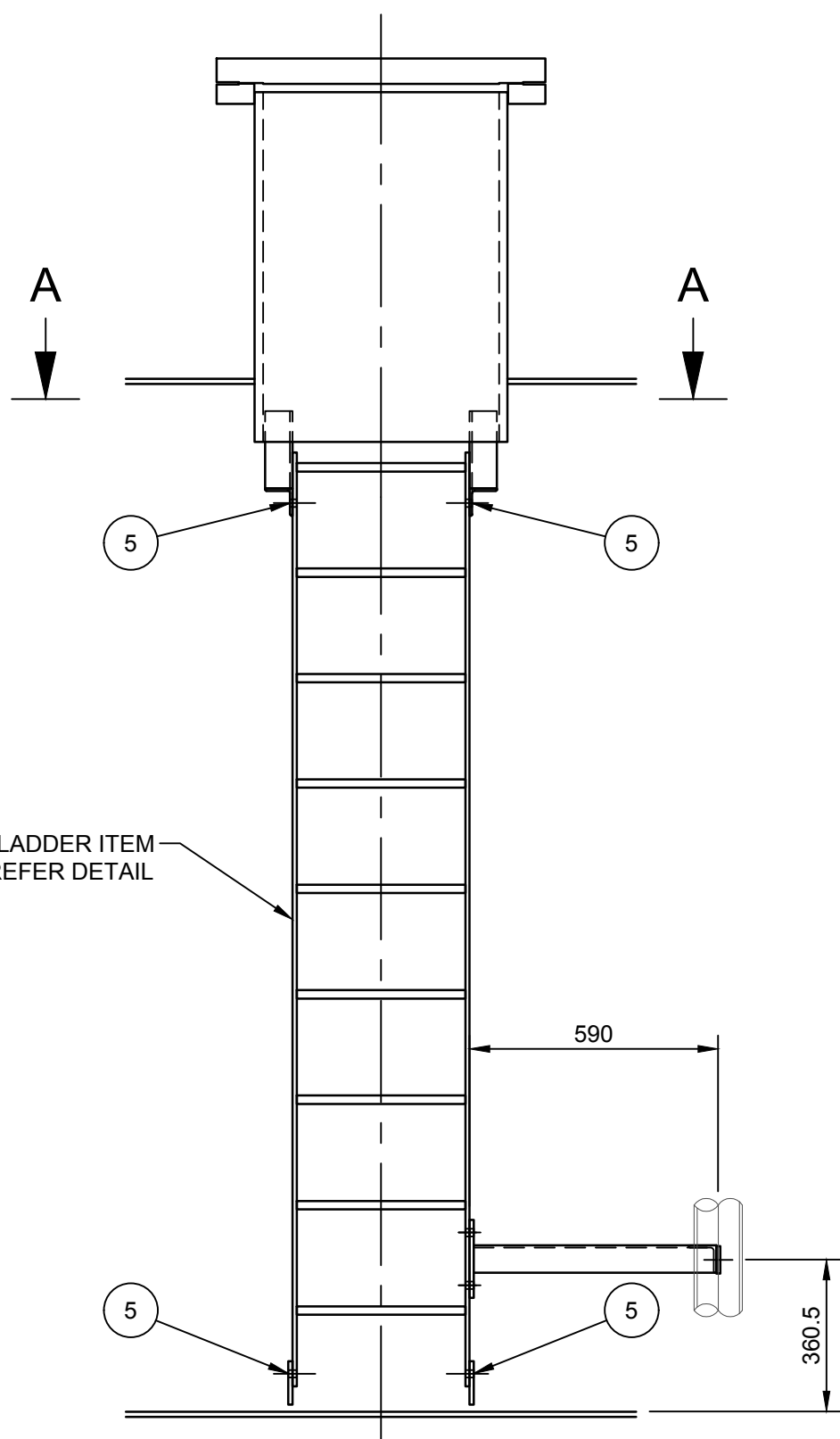
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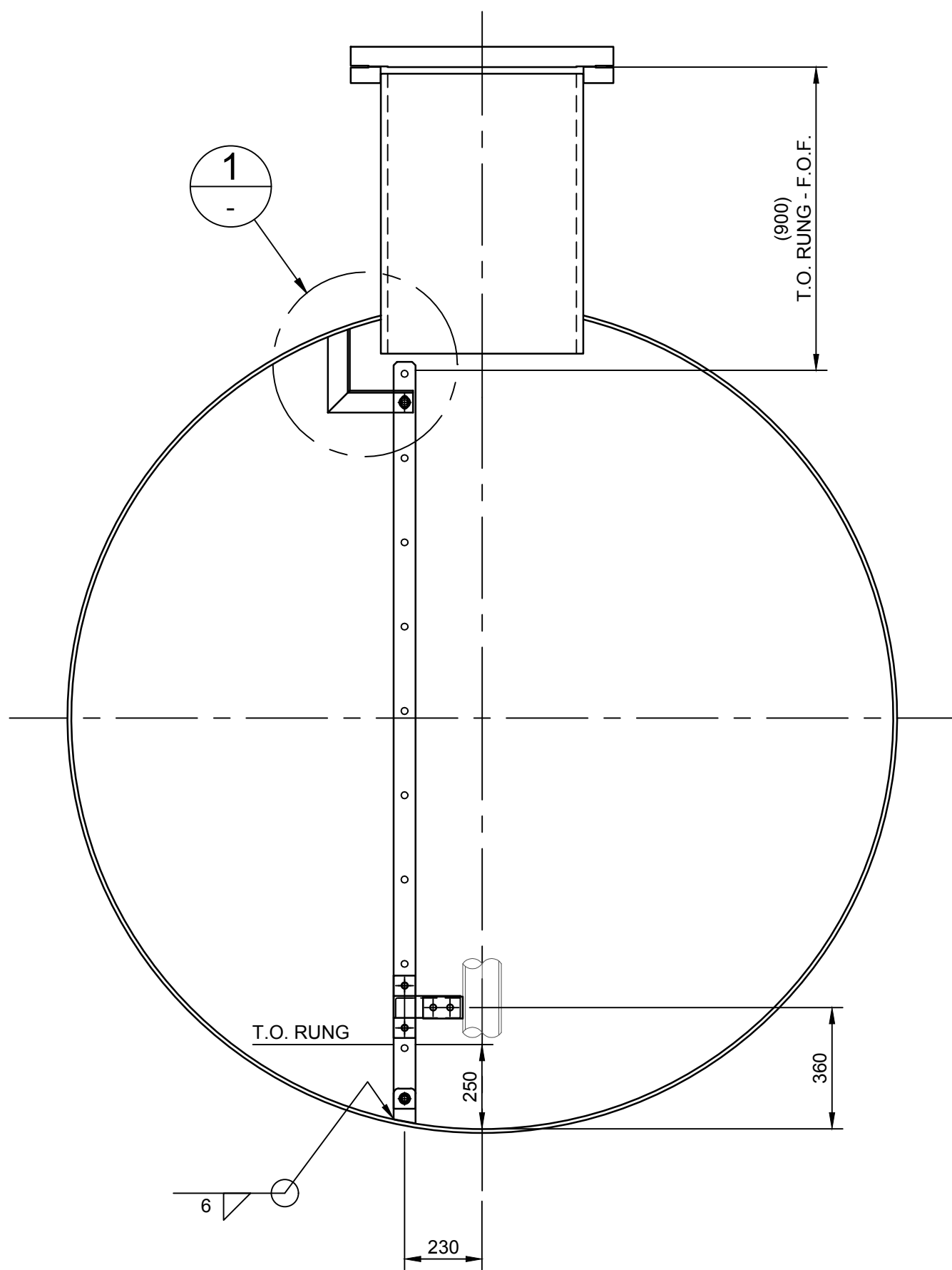
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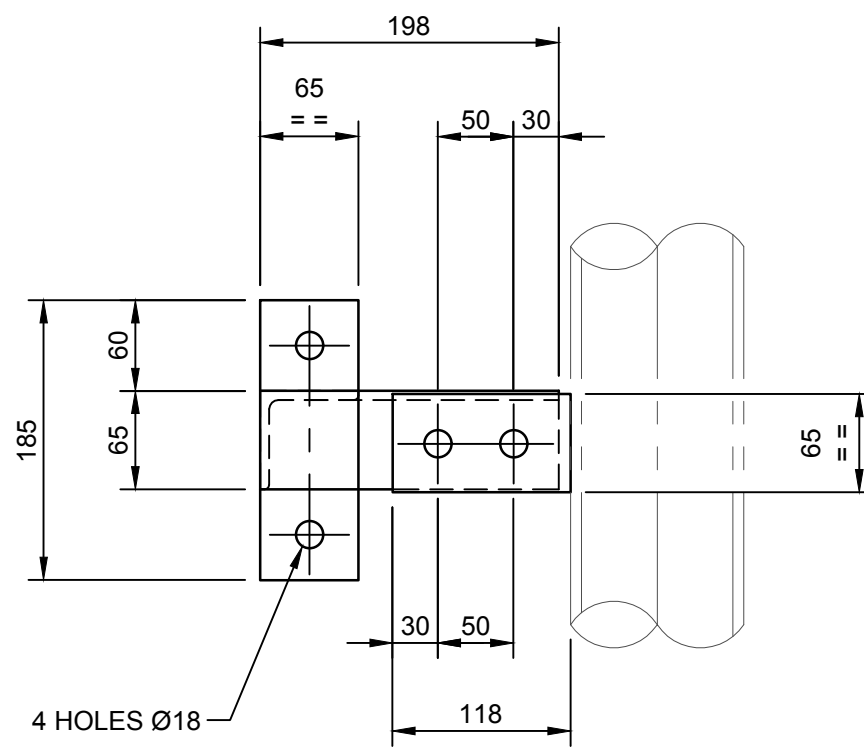
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SCALE 1:10



LADDER ASSEMBLY
SCALE 1:16



NOZZLE A PIPE BRACE
SCALE 1:5



VIEW B
SCALE 1:5

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9	BRACE	1	65 x 6 EA, 198 LG	AS3679.1-300	
8	BRACE	1	65 x 6 EA, 575 LG	AS3679.1-300	
7	BOLT PL.	1	65 x 10 FL, 185 LG	AS3679.1-300	
6	CLEAT	2	65 x 10 FL, 103 LG	AS3679.1-300	
5	BOLT / NUT	8	M16 x 45 LG Gr 4.6 BOLT C/W WASHER & NUT	AS1111 / AS1112	
4	BRACKET	2	65 x 6 EA, 248 LG	AS3679.1-300	
3	BRACKET	2	65 x 6 EA, 250 LG	AS3679.1-300	
2	RUNG	9	Ø20 RD BAR, 400 LG	AS3679.1-300	
1	STILE	2	65 x 10 FL BAR, 2258 LG	AS3679.1-300	
ITEM	DESCRIPTION	QTY	DIMENSIONS	MATERIAL	COMMENTS



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LADDER DETAILS
SUPAGAS
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160519

CAD FILE No:
LAP-3562

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AS SHOWN	T. O'DEMPSEY	12913 Q. GRYL	LAP-3562-05	1

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0	ISSUED FOR CONSTRUCTION	TOD	QG	QG	1/02/2017
REV	DETAILS	DRN	CHK	APP	DATE

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H

NOTES:

1. ALL LETTERING SHALL BE PERMANENT E.G. ENGRAVED & BLACK-INKED OR EMBOSSED (OR EQUIVALENT), MINIMUM 3mm HIGH

2. FOR NAMEPLATE MOUNTING BRACKET REFER TO DRG LAP-3562-03

175

165

Ø4.5

L & A Pressure Welding

EQUIPMENT #

T-001

DESCRIPTION

LPG U/GROUND VESSEL

DESIGNED TO

AS1210-2010 CLASS 1H

DESIGN PRESSURE

1550 kPa.g

DESIGN TEMPERATURE

0°C to 50°C

CONTENTS

L.P.G.

CORROSION ALLOW.

1.5 mm EXTERNAL

VOLUME - GROSS

105 800 L

VOLUME - PRODUCT 88%

93 104 L

AS4343 HAZARD LEVEL

B

HYDRO. TEST PRESSURE

2220 kPa.g

HYDRO. TEST DATE

MANUFAC. SERIAL No.

FINAL INSPECTION BY

FINAL INSPECTION DATE

DESIGN REGIST. No.

NO WELDING OR HEATING OF THIS VESSEL WITHOUT APPROVAL FROM AUTHORITIES.

175

185

0

ISSUED FOR CONSTRUCTION

TOD

QG

QG

1/02/2017

REV

DETAILS

DRN

CHK

APP

DATE

SCALE

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DRN

T. O'DEMPSEY

RPEQ

12913

Q. GRYL

LAP DRG No:

LAP-3562-06

REV

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NAMEPLATE

1

1.6mm SHT, 175 x 185

ASTM A240-316

ITEM

DESCRIPTION

QTY

DIMENSIONS

MATERIAL

COMMENTS

L & A

PRESSURE WELDING PTY LTD

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NAMEPLATE DETAILS

SUPAGAS

LPG UNDERGROUND VESSEL

T-001

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CAD DRAWING - DO NOT ALTER MANUALLY

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28 Crooked Billet Drive, Bridgewater

Stormwater Management Report – Rev 0

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PDA Contributors

Contribution	Name	Date
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Review 1	Rod Parsons	2025/06/30
Review 2	Mark Westerberg	2025/06/30

Revision History

Revision	Description	Date
0	First Issue	2025/06/30

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Abbreviations

AEP	Annual Exceedance Probability
ARI	Annual Recurrence Interval
ARR	Australian Rainfall and Runoff
BOM	Bureau of Meteorology
CC	Climate Change
DSG	Department of State Growth
IFD	Intensity - Duration - Frequency
IL-CL	Initial Loss - Continuing Loss
OSD	On-site Detention
SW	Stormwater

1 Project Overview

This report has been prepared by PDA Surveyors, Engineers & Planners to support the stormwater system design for the proposed development at 28 Crooked Billet Drive, Bridgewater. This report presents the stormwater management strategy for the proposed works, including assessment of pre- and post-development conditions, and outlines the proposed on-site detention measures designed to meet the relevant requirements.

1.1 Site Overview

The proposed works will be developed at 28 Crooked Billet Drive, Bridgewater, in the Bridgewater Industrial subdivision area, as shown in Figure 1. The site is currently occupied by Supagas, an industrial gas supplier. The land is zoned General Industrial.

The property has an approximate area of 3874 m² and is developed with two existing buildings, an office building and a shed, as well as a concrete slab area for general use, and a larger concrete pad where large gas tanks are placed. The remainder of the site consists predominantly of compacted gravel hardstand used for vehicle movements, gas storage, and general site access. Runoff from both roofed and hardstand areas is conveyed via stormwater pipes to an existing discharge point located at the north-eastern corner of the site, which drains to adjacent crown land.

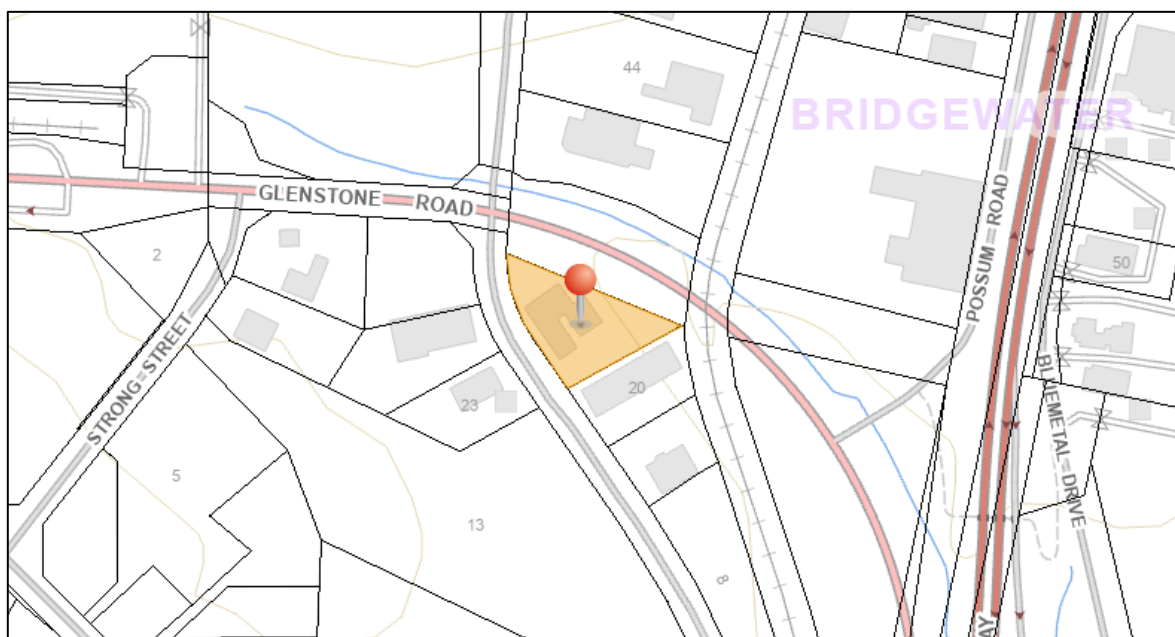


Figure 1 - Locality Plan

The proposed development involves the replacement of a section of existing gravel surface with a new concrete slab. This modification will result in an increase in impervious surface area, and as such, the Department of State Growth has outlined some requirements for stormwater discharge. These are discussed in Section 1.3.

The overall goal of this stormwater management plan is to propose a sustainable and effective stormwater system for this development. Following common practice, specific objectives include managing the surface runoff produced by new sealed surfaces, limiting flow to pre-development levels, and conveying them into approved discharge points.

1.2 Regulatory Framework

This stormwater system adheres to the Tasmanian Stormwater Policy Guidance and Standards for Development. In Tasmania, flow is managed through a major / minor storm approach. Design of stormwater systems are based on ARR 2019.

The minor drainage system maintains safety, manages nuisance, and minimises damage to property for frequent rainfall occurrences. Traditionally, Tasmanian Councils have set a range of minor network levels of service depending on the land use, as shown in Table 1. For this development, a 2% AEP level of service is considered applicable for the minor storm considering the site's industrial setting.

Table 1 - Minor system levels of service in Tasmania

ARI Minor - Residential	AEP	ARI Minor - Rural & Residential >2ha	AEP	ARI - Minor - CBD	AEP	ARI Minor - Business, Commercial and Industrial	AEP
1 in 5	20%	1 in 2	50%	1 in 50	2%	1 in 20	5%
1 in 10	10%	1 in 5	10%	1 in 20	5%	1 in 50 - Industrial	2%
1 in 20	5%	1 in 20	5%				

The major drainage system is primarily intended to mitigate risks associated with overland flows. The major system typically includes overland flow paths on roads and through open space. This system conveys additional runoff produced during larger, infrequent events which are potentially hazardous due to the velocity and depth of flows.

Levels of service of major systems are conventionally designed to convey the 1% AEP event, occasionally with consideration for an additional CC loading depending on conditions stated on the permit and the local government requirements.

1.3 Requirements for Stormwater Detention

The proposed development involves the replacement of a section of existing gravel surface with a new concrete slab. This modification will result in an increase in impervious surface area, and as such, the Department of State Growth has requested the following, via email, on the 1 October 2024:

- The proposed stormwater works should utilise any current point of discharge for the site (it is assumed that the existing DN150 discharges to the table drain) i.e. there should only be **one** point of discharge for the site.
- The stormwater discharge should not increase from pre-development levels, and therefore on-site stormwater detention may be required.

2 Stormwater Quantity Management

The stormwater system has been modelled in Drains and its design criteria and assumptions are discussed in the following sections.

2.1 Design Criteria

- The minor storm level of service is set for the 2% AEP (1 in 50 years);
- The major storm is set for the 1% AEP (1 in 100 years);
- Restrict runoff to pre-development levels for both 2% and 1% AEP events.

2.2 Hydrological Model

A full range of storm durations from 5 minutes to 2 hours was simulated using the Extended Rational Method (ERM) hydrological model. This method allows multiple storm simulations within the rational method framework. A different runoff coefficient has been assumed depending on the surface material (e.g., roof, concrete, and gravel). Table 2 shows the assumed runoff coefficients for the 10-year ARI storm (C10), along with the corresponding coefficients for the 1 in 50-year (C50) and 1 in 100-year storm (C100). The C10 coefficients are adjusted using frequency conversion factor (Fy) specific to each return period. These factors are shown in Table 3.

Table 2 – Runoff coefficients

Parameter	C10	C50	C100
Impervious Area C10 (roofed and concrete areas)	0.9	1.0	1.0
Gravel Area C10	0.5	0.58	0.6

Table 3 - Frequency Conversion Factor

ARI (years)	1 and <1	2	5	10	20	40	60	80	100
Fy	0.8	0.85	0.95	1.0	1.05	1.13	1.17	1.19	1.2

Other inputs considered for the SW network design include:

- The use of the 2016 IFD data from the BOM;
- Ten temporal patterns obtained from the ARR Data Hub for each duration;
- Median results for the ensemble of storms using the specified parameters will be obtained.

This approach aims to provide a comprehensive examination of the numerous possible rainfall scenarios. However, it is important to note that weather conditions may vary from those typically assumed and recommended by the ARR.

2.3 Pre-development Scenario

2.3.1 Sub-catchment delineation

The pre-development sub-catchments are defined depending on the different surface types within the catchment, namely gravel, and impervious (e.g., roof and concrete), as shown in Figure 2. In this figure, the yellow area represents concrete, the blue area represents roof areas, and the remaining is considered as gravel. A summary of these catchments is presented in Table 3.



Figure 2 – Pre-development catchments

Table 4 - Summary of Pre-development Sub-catchments

Sub-catchment	Area, m ²
Roof	809
Gravel (to be concreted)	666
Gravel (to remain)	2049
Exist concrete	185
Gas Tanks	165
TOTAL	3874

2.3.2 Estimation of Pre-development Peak Discharge

A Drains model was developed to represent the pre-existing conditions and median flows were obtained. The estimated pre-development peak flows during the relevant storm events (i.e., 2% and 1% AEP events) are shown in Table 5. Reference is made to Annexure B, where results from the model are shown.

Table 5 - Pre-development peak flows

AEP	Peak flow (l/s)
2%	65
1%	73

2.4 Proposed Stormwater System

The proposed stormwater management strategy has been designed to ensure post-development discharge rates remain below pre-development levels, in accordance with the requirements of the Department of State Growth (DSG). The strategy addresses the increase in impervious area resulting from the replacement of existing gravel surface with new concrete hardstand.

The runoff from the office building roof will be directed into two rainwater tanks – one with a capacity of 4000 L and the other 5000 L - installed adjacent to the building. The tanks will provide temporary storage of roof runoff and gradually release it at a controlled rate to compensate for the increased runoff generated by the additional concrete areas. This approach ensures that the total site discharge remains below pre-development conditions during storm events.

Runoff from the new concrete slab and other surface areas will be collected and conveyed via surface grading and drainage pits to a DN225 stormwater pipe. This pipe discharges to the existing outlet located along the northern boundary of the site, where runoff enters land managed by DSG. The proposed discharge has been designed to comply with DSG requirements, ensuring that post-development flows do not exceed pre-development conditions.

The combination of on-site detention via the rainwater tanks and the dedicated drainage infrastructure will ensure effective stormwater management for the site, mitigating any potential impacts on downstream properties or infrastructure.

Reference is made to Annexure A, where the proposed SW system is shown.

2.5 Post-development Scenario

2.5.1 Post-Development Sub-catchments

In the post-development scenario, the site layout largely remains consistent with the pre-development conditions. The change in the post-development condition is the replacement of a portion of the existing gravel surface with a new concrete slab. This modification increases the impervious area, resulting in an increase of volume and peak flow generated during storm events.

2.5.2 Estimation of Post-development Peak Discharge

Hydrologic modelling was undertaken to assess the resulting peak flows under the 1% and 2% AEP events. Results indicate a minor increase in runoff volume and peak discharge due to the change in the surface type. However, this increase is effectively attenuated by the inclusion of on-site detention measures described in Section 2.4. Results for peak flow under the 2% and 1% AEP storm events are shown in Table 6.

Table 6 - Post-development peak flows with OSD measures

AEP	Peak flow (ℓ/s)
2%	58
1%	70

3 Summary

An overview of the stormwater management plan for the proposed industrial development is provided in this section:

- The report supports the design of the stormwater system for the proposed works at 28 Crooked Billet Drive, Bridgewater;
- Pre-development flows are estimated as **73 ℓ/s** for the 1% AEP, and **65 ℓ/s** for the 2% AEP;
- The proposed stormwater system, consisting of 2 RWTs collecting stormwater from the office building roof, is designed to limit post-development peak flows to pre-development levels as requested by DSG;
- Post-development flows are successfully limited below pre-development levels. Please refer to Table 7 for a comparison between peak flows;

For further reference, please see concept plans (54479MW-ENG-REV P2, dated 27/06/2025) also submitted.

Table 7 - Comparison between pre-development and post-development flows

Discharge from subdivision	Pre-Development (ℓ/s)	Post-Development with OSD measures (ℓ/s)
2% AEP	65	58
1% AEP	73	70

4 Qualifications

This document is intended solely for the purpose of documenting the calculations followed for the design of the stormwater drainage system and should not be used for any other purpose without proper consultation. The results presented here reflect median outcomes based on various combinations of probable rainfall characteristics. These should not be understood as deterministic predictions of the stormwater system's performance, as they are influenced by both epistemic (arising from incomplete knowledge) and aleatory (arising from the inherent variability of natural systems) uncertainties associated with stormwater and hydrological models. It should be used by qualified professionals in conjunction with their professional judgement.

The calculations presented in this document are based on the data available at the time of writing. The design calculations were performed in accordance with the relevant design standards and guidelines applicable at the time of the writing. Future updates to these standards may require revisions to the design.

This document contains several assumptions regarding the site's condition, hydrological parameters, and projections for future climate scenarios (when required). These assumptions are detailed within the report and any deviation from these assumptions may affect the outcome of the design. The design is based on site-specific conditions understood from available information. This includes information and data provided by third parties. While this information is believed to be accurate, no warrant is given as to its completeness and or reliability.

It is important to acknowledge that many stormwater treatment assets, including GPTs, are subject to ongoing audits and field evaluations, which continue to enhance understanding of their actual performance in real-world conditions. As this knowledge evolves, previously unidentified limitations or inefficiencies in certain products may become apparent. While every effort has been made to specify systems aligned with current best practices, the performance of these assets ultimately depends on the product's engineering design, proper installation, effective operation, and regular maintenance—factors that lie beyond the scope of this design.

While every effort has been made to ensure the accuracy of the calculations and design, PDA and the authors assume no liability for any direct, indirect, or consequential damages resulting from the use of this document. The performance of the stormwater drainage system is subject to external factors such as extreme weather events, future land-use changes, and maintenance practices. These factors can significantly impact the effectiveness of the system.

Annexures

ANNEXURE A – Proposed SW System

ANNEXURE B – Results from Drains model

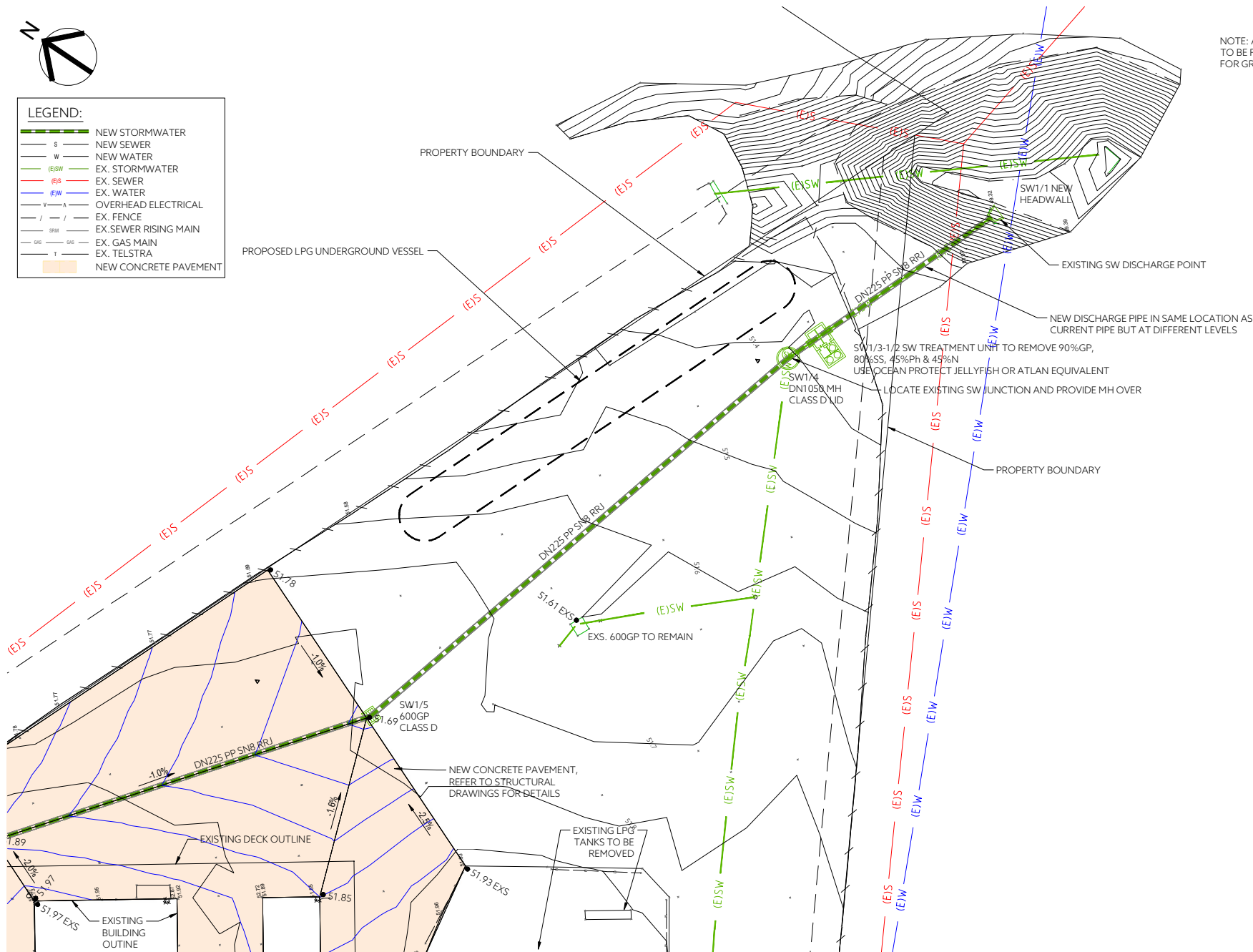
ANNEXURE C - 2016 IFD Tables from BOM

ANNEXURE A – Proposed SW system



LEGEND:

- NEW STORMWATER
- S NEW SEWER
- W NEW WATER
- EX. STORMWATER
- (E)S EX. SEWER
- (E)W EX. WATER
- V A OVERHEAD ELECTRICAL
- / / EX. FENCE
- SM EX. SEWER RISING MAIN
- GAS EX. GAS MAIN
- T EX. TELSTRA
- NEW CONCRETE PAVEMENT



NOTE: ALL NEW AND EXISTING STORMWATER PITS TO BE FITTED WITH 200 MICRON MESH SCREENS FOR GROSS POLLUTANT TRAPPING.

REV	AMENDMENTS	DRAWN	DATE	APPR.	DRAWING STATUS
-	-	-	-	-	FOR PLANNING APPROVAL
-	-	-	-	-	COORDINATE/DATUM: PLANAR
-	-	-	-	-	THIS SHEET MAY BE PRINTED USING COLOUR AND MAY BE INCOMPLETE IF COPIED

DESIGNED: SC	REVIEWED: MW
DRAWN: SC	REVIEWED: MW
JOB MANAGER: MARK WESTERBERG	ISSUED DATE: 27/06/2025

CLIENT: DDM CIVIL - SUPAGAS
PROJECT DESCRIPTION: PAVEMENT LEVELS AND DRAINAGE
ADDRESS: 28 CROOKED BILLET DRIVE, BRIGHTON
DRAWING TITLE: SITE PLAN 2

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	JOB NUMBER 54479HC	DISCIPLINE C	SHEET 201	REVISION P2
	REGISTRATION NUMBER: ----			

ANNEXURE B – Results from Drains Model

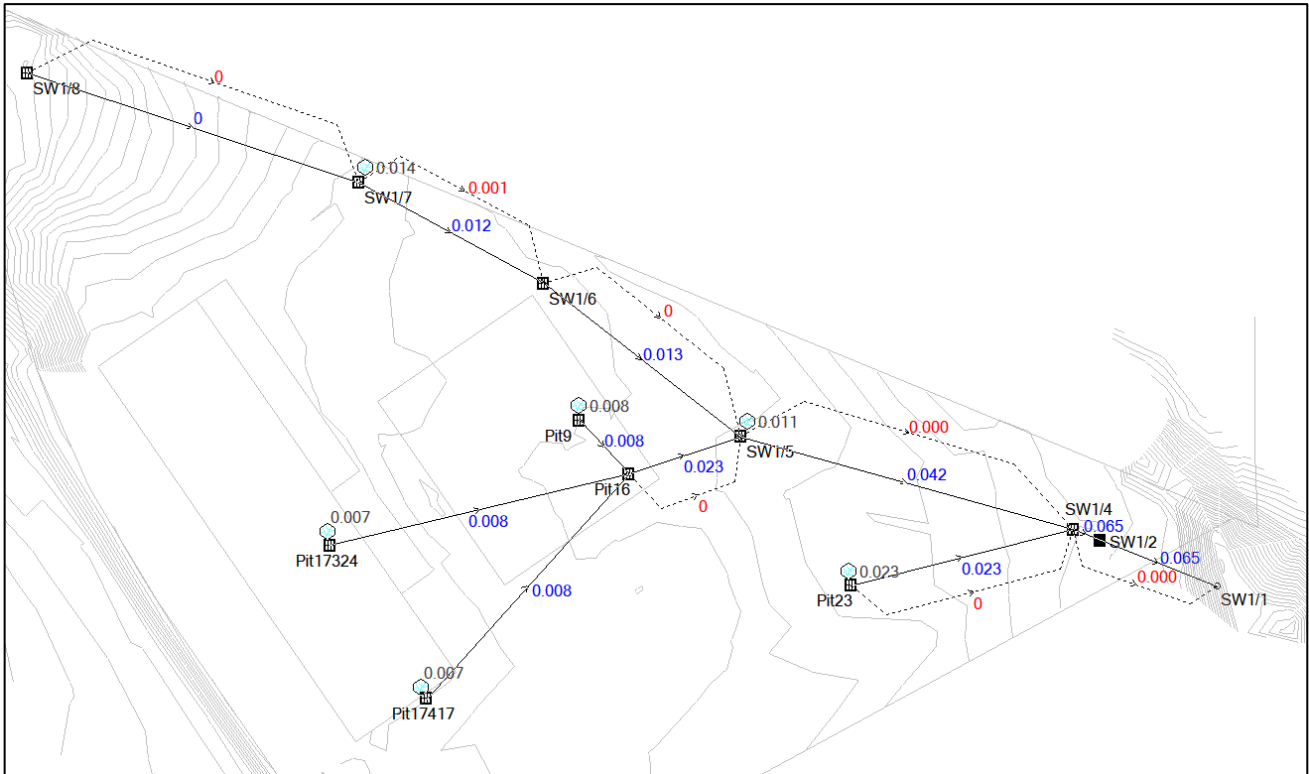


Figure C. 1 - Model results for 2% AEP Pre-development

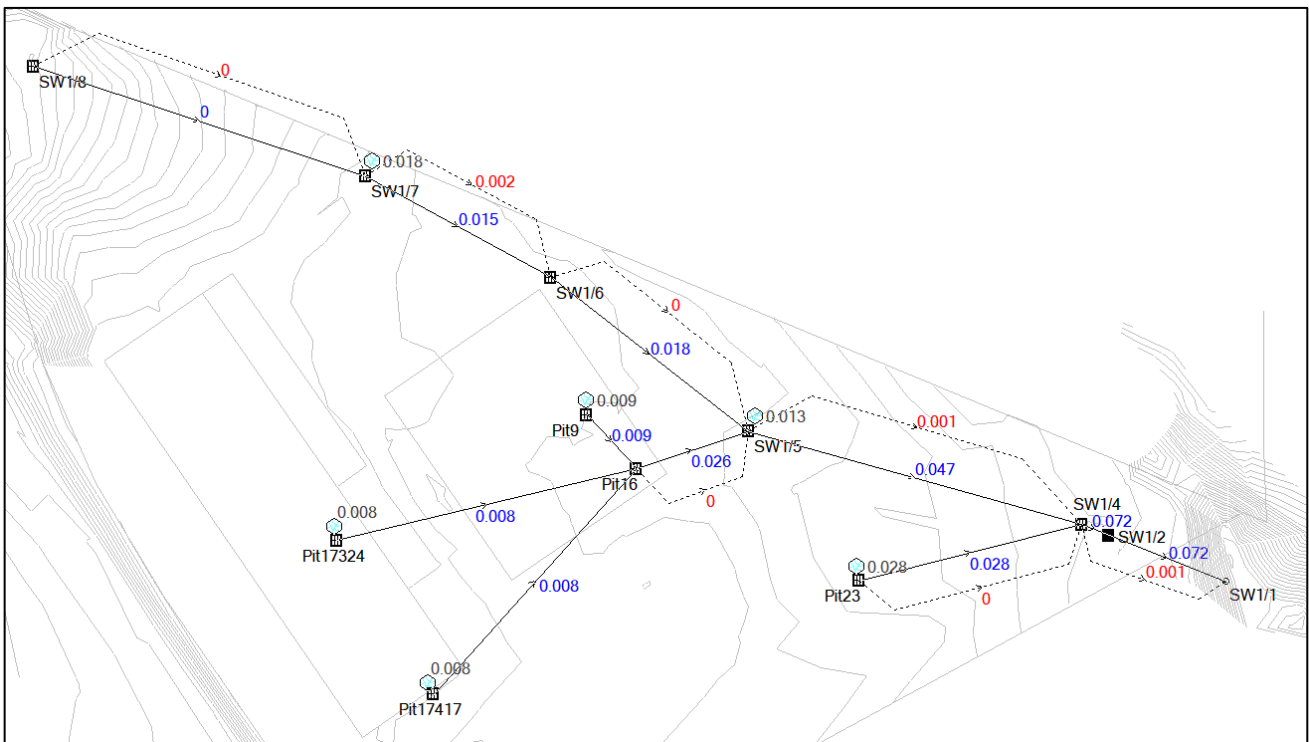


Figure C. 2 - Model results for 1% AEP Pre-development

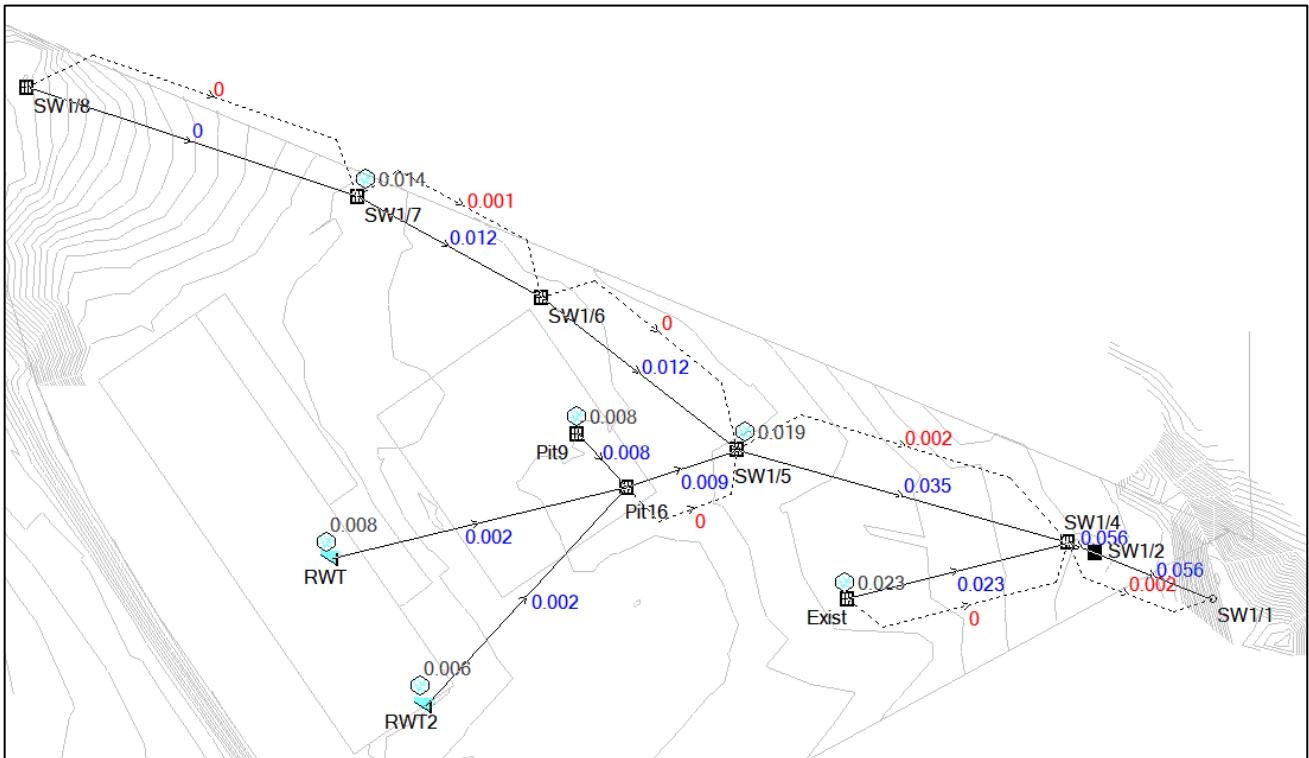


Figure C. 3 - Model results for 2% AEP Post-development

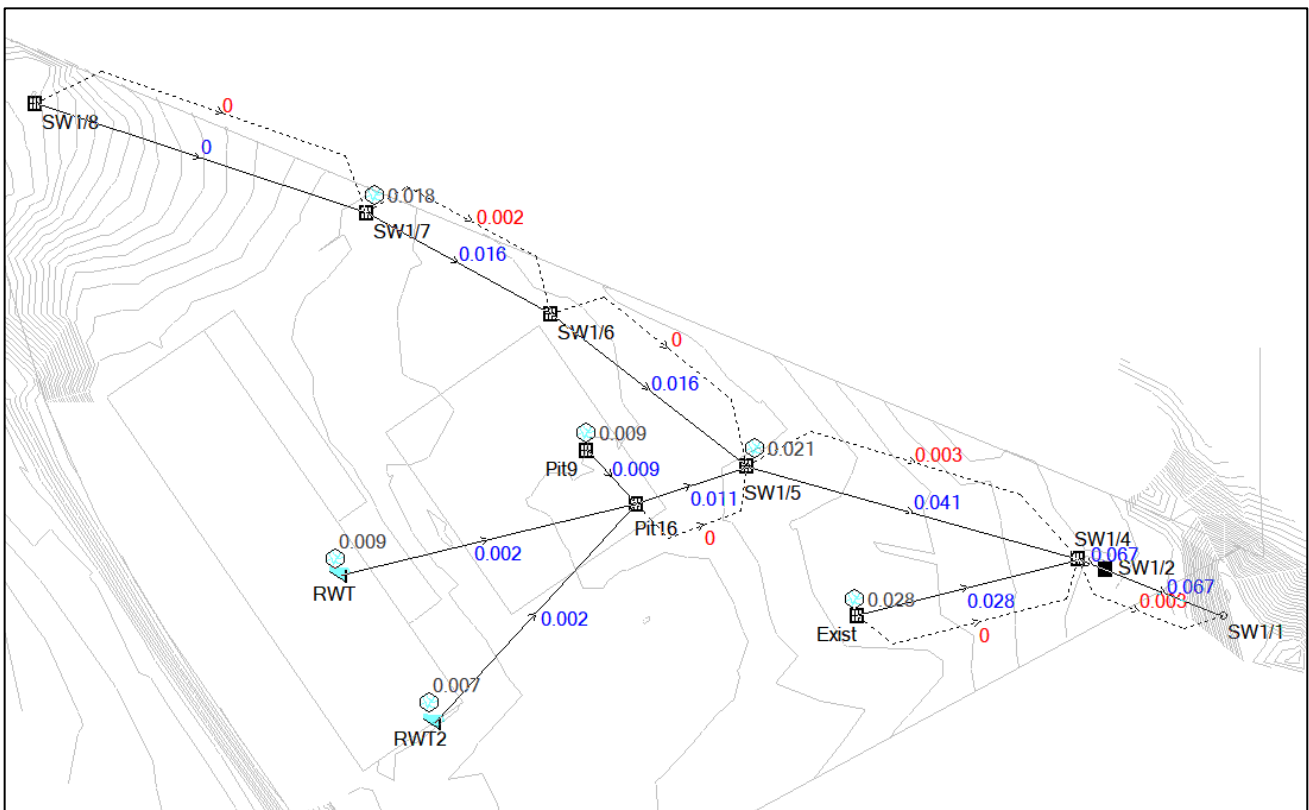


Figure C. 4 - Model results for 1% AEP Post-development

ANNEXURE C – 2016 IFD Table from BOM

<i>Duration</i>	<i>63.20%</i>	<i>50%</i>	<i>20%</i>	<i>10%</i>	<i>5%</i>	<i>2%</i>	<i>1%</i>
<i>1 min</i>	0.99	1.12	1.57	1.91	2.25	2.75	3.16
<i>2 min</i>	1.71	1.93	2.63	3.13	3.63	4.24	4.7
<i>3 min</i>	2.26	2.56	3.51	4.19	4.88	5.76	6.43
<i>4 min</i>	2.71	3.07	4.25	5.1	5.97	7.12	8.04
<i>5 min</i>	3.1	3.51	4.89	5.88	6.92	8.34	9.49
<i>10 min</i>	4.5	5.11	7.19	8.74	10.4	12.8	14.8
<i>15 min</i>	5.47	6.22	8.76	10.7	12.7	15.7	18.2
<i>20 min</i>	6.25	7.1	9.98	12.1	14.4	17.8	20.6
<i>25 min</i>	6.91	7.85	11	13.4	15.8	19.4	22.4
<i>30 min</i>	7.5	8.51	11.9	14.4	17	20.8	23.9
<i>45 min</i>	8.96	10.2	14.1	17	19.9	24.1	27.4
<i>1 hour</i>	10.2	11.5	15.9	19	22.2	26.6	30
<i>1.5 hour</i>	12.2	13.7	18.9	22.4	26	30.6	34.3
<i>2 hour</i>	13.8	15.6	21.3	25.2	29.1	34.1	37.9
<i>3 hour</i>	16.5	18.7	25.4	29.9	34.3	39.9	44.2
<i>4.5 hour</i>	19.7	22.3	30.4	35.7	40.7	47.3	52.3
<i>6 hour</i>	22.2	25.2	34.4	40.4	46.1	53.8	59.5
<i>9 hour</i>	26.2	29.8	40.9	48.2	55.1	64.6	71.9
<i>12 hour</i>	29.3	33.4	46	54.4	62.4	73.6	82.3
<i>18 hour</i>	33.8	38.6	53.6	63.8	73.7	87.8	98.9
<i>24 hour</i>	37	42.3	59.1	70.7	82.2	98.6	112
<i>30 hour</i>	39.5	45.2	63.3	76	88.7	107	121
<i>36 hour</i>	41.5	47.4	66.6	80.2	93.9	113	129
<i>48 hour</i>	44.4	50.7	71.4	86.3	102	123	140
<i>72 hour</i>	48.2	54.9	77.2	93.5	110	133	151
<i>96 hour</i>	50.7	57.6	80.6	97.3	115	138	156
<i>120 hour</i>	52.6	59.6	82.9	99.7	117	140	158
<i>144 hour</i>	54.2	61.4	84.7	101	118	140	159
<i>168 hour</i>	55.7	63	86.3	103	119	141	159

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