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Our Ref: 251013

8<sup>th</sup> November 2024

Update 19 November 2024

Update 31 January 2025

Holmes Dyer Pty Ltd Leve 3, Reid House, 15 Featherstone Place Adelaide SA 5000

#### **ATTENTION: S Holmes**

Dear Stephen

#### SITE ANALYSIS REPORT FOR INFRASTRUCTURE REQUIREMENTS - BOYER PSP

I am writing to you about the findings to date for the provision of infrastructure services to the proposed subdivision based on the Plan 1792-002 R2>14.01.25 provided by Holmes Dyer. This working document shows approximately 362 lots. The essential services required include power supply from TasNetworks, sewer systems and water supply from TasWater and communications from NBN Co. Brighton Council have provided feedback on stormwater management including stormwater quantity and quality control. Consideration has been given to the provision of natural gas supply from TasGas. Additionally, to the infrastructure requirements, information from Tas Irrigation is included showing the approximate location of an irrigation supply pipeline. Keith Midson Traffic Engineer, has provided intersection treatments for entry points along Boyer Road.

The collection of background information on the services will inform the establishment of funding models.

#### TasNetworks - Power Supply

#### Contact Person - Gary Hancl - 0438 338 060 Alan Heald and Alex Izbecki

Initial contact was made via email to Clodagh Doyle on 24/9/24. Due to some internal staff movements, setting up an Early Engagement meeting proved difficult. On 31/10/24 a discussion finally took place with Gary Hancl – Team Leader. This was a good opportunity to explain the project context and that we are planning the future and not the developers of the project. He understands that we are looking for high level design of the system that will lead to high level costings enabling a model for funding to be established that links the power supply infrastructure to the future development. He has now organised an Early Engagement meeting for 10am on 12/11/24 with the right people within TasNetworks. Further reporting to follow.

#### Update 19/11/24

Early Engagement meeting was held on 12/11/24 with Gary Hancl, Angela Trewin and Chris Symons. The Network Planning Representative was not at the meeting but was contacted after the meeting. The positive outcomes were that TasNetworks do not have any issues in terms of supply to this location. The design may





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include a looped supply especially if there is connectivity to Cobbs Hill Road. The internal requirements for supply are a 750kVa sub station per 100 lots. Preferable substation locations are central to the lots being supplied.

# <u>Update 10/1/24</u>

After advice received from the Enquiry by Design Meeting from TasNetworks, the suggestion was to apply for a Feasibility Study. This occurred and on return to the office 6/1/25 and invitation for a new Early Engagement Meeting was received for 9/1/25 from different people in the Negotiated Connections Team, including Alan Heald and Katherine Johnstone. At the meeting Alex Izbecki was in attendance and understood that we are in a pre-planning and pre-subdivision phase.

The machinations of TasNetworks do not cater for pre-planning and they are used to dealing with subdivision developments with Planning Approval. They could do the feasibility study for a cost and would require a Staging Plan, which has now been produced.

Having said this they have indicated that they have an obligation to supply, that any costs associated with augmentation or upgrade requirements are bundled up into the developer costs that are normally associated with a subdivision. These developer costs include the HV lead in, sub-stations and underground reticulation. TasNetworks have indicated an indicative cost of \$15 000 to \$20 000 per lot.

Minutes of the meeting attached in Appendix D.

# Funding of Infrastructure Decision 31/1/25

Based on advice from TasNetworks the design and supply of the power to the future subdivision site is achievable. Augmentation of HV supply is achievable and final design will be developed with a development plan as per TasNetworks practice. The costs associated to achieve the required HV supply and the breakdown to LV via substations will be distributed to the developer generally as per lot basis. No costs are to be included in a contribution's arrangement.

# TasWater – Sewer Systems and Water Supply

# Contact Person - Elio Ross - 0467 874 330 Update provided 22/11/24

# Sewer System

In the first instance the advice from TasWater was to lodge a Service Enquiry. This was lodged with TasWater through their Portal on 16/10/24. To provide context to the servicing discussion we made calculations for Equivalent Tenements (ET's) for the sewer based on the three natural catchments within the development area. Each catchment has a low point adjacent to Boyer Road and will require a Sewer Pump Station. Pump Station 1 is located in 50 Boyer Road (Property ID: 7676361, Title Ref: 44724/8). Pump Station 2 is located in 170 Boyer Road (Property ID: 7676396, Title Ref: 44724/9) and Pump Station 3 is located furthest west in the property known as Boyer Road (Property ID: 1972194, Title Ref: 44724/2). The ET's for each catchment and Pump Station are listed in Table 1. A typical allowance for a 20% growth rate has also been included.





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The sewer pump stations have been sized at the highest level for environmental sensitivity based on their close proximity to the Derwent Estuary, meaning the storage time will be 8 hours (in accordance with *Taswater Supplement to WSA 04-2005 2.1 WSAA Sewage Pumping Code of Australia*) and hence the pump station size will be larger than a regular pump station with low environmental rating. The default environmental rating may be lowered subject to further risk assessments in consultation with Taswater, however due to the proximity to the Derwent Estuary, it is assumed that a high rating is appropriate and is likely to be required.

Consideration was given to a common rising main to allow each pump station to discharge into a common line before being discharged into a gravity manhole outside 24 Boyer Road. A professional discussion took place to investigate the possibility of achieving this outcome. After this discussion it was determined that it would not be possible to achieve this. There are two other possible ways to achieve the discharge requirements of the three pump stations. The first is to install an individual rising main to each sewer pump station. This means a common trench can be used from near Serenity Drive all the way to 24 Boyer Road. The other possible solution is to daisy chain the pump stations, that is one pumps to the other and then the next one. This means the pump stations would have to increase in size to accommodate the additional effluent discharged by the previous pump station and the inflow from the local catchment. A high-level cost analysis of these options will be carried out. In the first instance the multiple rising main option is likely to be the best option for cost but also for the fact that the stand-alone development can occur on the individual larger parcels. Please see new advice received about the pump stations and a common rising main.

The sewer pump station volumes for the multiple rising main option are shown in Table 1.

Pump Station ID	Developed Equivalent Tenements (ET's) (+20% Growth Rate)	Pump Station Volume	Pump Flow Rate
1 (50 Boyer Road)	134	20.1 m <sup>3</sup>	10.62 L/s
2 (170 Boyer Road)	248	37.2 m <sup>3</sup>	18.04 L/s
3 ('Boyer Road')	71	10.6 m <sup>3</sup>	5.95 L/s

Table 1 – Sewage Pump Station Parameters – Current Plan > 362 lots

Preliminary comment from Taswater was received on 05/11/24 (included in Appendix B), with more detailed advice from Taswater's assets and modelling teams still to follow. Taswater's preliminary comment assumed connection to the existing Riverside Drive Sewage Pumping Station (ID: GRPSP12) which represents another option for connection location, however indicated that several upgrades to the existing piped network and sewage pumping station would likely be required in this instance and the Riverside Drive SPS will not be included.

#### Service Enquiry Update 22/11/24

After discussion with TasWater and receiving the written advice indications are that a common rising main linking all three pumpstations is a possibility. This may change the configuration of each of the pump stations and will be subject of future design. The already calculated total capacity requirements for all three pump stations are unlikely to change and hence these sizes can be used to calculate costs in relation to a contribution model.





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TasWater have also provided additional information about their downstream sewerage infrastructure. They are indicating that there is significant strain on the gravity system, the existing sewer pump stations and rising mains. One positive is that the Green Point Sewerage Treatment Plant has adequate capacity to cater for the development.

Gravity main upgrades are required between the receiving manhole and the Nielsen Parade SPS. Further to this gravity and rising main upgrades maybe required from the Nielsen Parade SPS to the Green Point STP.

The Sewer Pump Station at Nielsen Parade requires an upgrade in volume, at total of 52.6kL. Taswater water have nominated that 30.1kL are the responsibility of the development. Further to this pump upgrades will be required with additional flow rate. The additional flow rate will require a larger rising main. These works are shown on Plan 251013 C503, with alternatives for the possible rising main upgrades. TasWater Servicing Advice found in Appendix B.

#### Funding of Infrastructure 31/1/25 - Sewer

The upgrade to the existing Nielsen Parade Pump Station will have shared costs between TasWater and an infrastructure contribution for Boyer PSP. Taswater have been clear in regard to volume upgrades and pump upgrades and their portion of these costs. The final part of the puzzle is the final rising main size. The cost difference is minimal in the rising main pipe diameter as most of the cost is in trenching and backfilling. A BoQ has been produced to assist the Quantity Surveyor to establish the apportioned costs to the contributions plan.

#### Water Supply

The creation of a reticulated water supply network to service the development will require connection to the existing Taswater supply network. The closest connection location is at the intersection of Serenity Drive and Boyer Road to an existing DN100 PVC main. Preliminary comment from Taswater (included in Appendix B) has indicated that the existing DN100 main will be too small to service the development with a DN200 connection (or 2 x DN150) typically required for a development of this size. The existing Taswater network within a reasonable proximity to the site (developed areas to the east) consists entirely of DN100 or less sized mains, meaning no suitable connection is in close proximity to the site from this area.

The existing developed area to the east is supplied from the single Bridgewater Reservoir (ID: HOBWS017), which is located adjacent to Cobbs Hill Road to the north of the site. Taswater has advised that connection for the site will likely be required directly from the existing DN375 main (ID: A203814) at the reservoir. Connection at this location would mean the new water supply will be required to run through areas of 29 Cobbs Hill Road not previously included for development. Taswater has also advised that the creation of a new road reserve would be required to house the new DN200 main as new water mains cannot be located within private property. It is likely that the new mains servicing the site will also require connection back into the existing network at Serenity Drive to create a closed loop.

No other existing infrastructure suitable for direct connection is present within the surrounding areas. Several large bulk transfer mains are located further north of the site as well as a DN630 irrigation main that partially runs through 50 Boyer Road, however these are not suitable nor available for connection. This means that Taswater's initial comments on connection location nearby the reservoir are very likely to be the required solution for the site.





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In preliminary assessment of possible connections to Serenity Drive, an EPANET model has been undertaken which has been adjusted to include supply from the Bridgewater Reservoir. Preliminary results from the model show that adequate pressures and flows are expected to be available to all lots on the site inclusive of both residential and fire flows to the most hydraulically disadvantaged lot. A screenshot of the EPANET model is provided in Figure 1 below.

#### Service Enquiry Update 22/11/24

#### Water Infrastructure

Previous advice in regards to supply from the reservoir on Cobbs Hill Road still applies including the trunk supply at DN200 then spreading to DN100 throughout the subdivision. The new advice is that the capacity of the reservoir is not adequate to maintain existing servicing and supply the proposed subdivision. To this end TasWater will require additional capacity, either by rebuilding the existing reservoir with additional capacity or by building a second reservoir to give the additional capacity. If there is adequate land area a second reservoir is the likely outcome as this will limit supply disruption. We will discuss further with TasWater about the like volume requirement for additional storage.



Figure 1 – EPANET Model (Cobbs Hill Road Connection)

This model provides DN 200 lead in from the reservoir spreading to a DN100 network within the subdivision.





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# Funding of Infrastructure 31/1/25 - Water

The requirement for additional reservoir storage at Cobbs Hill Road will fall under an infrastructure contribution model. Also included will be the DN200 connection from the Cobbs Hill Reservoir into the subdivision as it is required. Link water infrastructure that enables 50 Boyer Road, 170 Boyer Road and the third property on Boyer Road to be included in the infrastructure contribution model. A Bill of Quantities has been prepared for use by the Quantity Surveyor.

# Council - Stormwater Discharges and Onsite Detention - Internal Roads

# Contact Person – Leigh Wighton – 0418 569 044

A meeting with Brighton Council was undertaken on 05/11/24 which included discussion on stormwater quality and quantity management with the information provided below being in-line with these discussions and Council's direction/requirements.

The land has three overall catchments with each falling to Boyer Road as shown on the concept plans provided in Appendix A. Each catchment discharges via an existing pipe under Boyer Road. Catchment 1 (50 Boyer Road) has a twin DN375 discharging under Boyer Road into a table drain that runs towards the railway crossing point at Riverside Drive. Catchment 2 (170 Boyer Road) discharges via a DN900 under Boyer Road and then under the railway corridor and finally to the Derwent Estuary. Finally, Catchment 3 ('Boyer Road') has a DN1200 discharging under Boyer Road and then under the railway corridor to the Derwent Estuary. Photos of the existing pipes from the upstream side are provided in Appendix C.

The Catchments 2 & 3 have defined gullies, and because of the rural nature have farm dams capturing flows at various locations. While these dams provide online storage, this has been ignored for the overall catchment modelling. The catchment modelling was carried out using the software package DRAINS to determine the impact of increasing the impervious areas due to development. A screenshot of the DRAINS model is provided in Figure 2. The discharge parameters used consisted of maintaining the existing pipe sizes beneath Boyer Road to control flows up to the 1% AEP event including a climate change increase factor of 16.3%. The results of the modelling including likely detention sizes required, are shown in Table 2, noting that catchment 3 did not require any detention with the existing DN1200 pipe and roadside depression being adequate to build driving head and discharge of the post-development flows.

Catchment ID	Existing Pipe Discharge Size	Peak 1% AEP Post- Developed Flow (Generated by Catchment)	Approximate Detention Volume Required	Peak 1% AEP Post- Developed Flow Through Culvert (With Detention)
1 (50 Boyer Road Discharge)	2 x DN375	1.04 m <sup>3</sup> /s	1,300 m <sup>3</sup> (Approx. 1400m <sup>2</sup> , 2m max. depth)	0.358 m³/s
2 (170 Boyer Road Discharge)	DN900	3.57 m³/s	2,360 m <sup>3</sup> (Approx. 2000m <sup>2</sup> , 3m max. depth)	2.23 m³/s

Table 2 – Stormwater Flows and Detention Requirements





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3 ('Boyer Road'	DN1200	3.56 m <sup>3</sup> /s	Nil (sufficient pipe	3.35 m <sup>3</sup> /s
Discharge)			capacity/inlet	
			storage)	

With the presence of an existing gully within Catchment 2, the topography is expected to lend itself to the creation of a suitable detention basin. The topography adjacent to the outlet for Catchment 1 is less well defined in regards to an existing low point/water course and would likely require additional earthworks to construct a suitable detention basin.



Figure 2 – DRAINS Model (1% AEP Event)

#### Funding of Infrastructure

It is likely that the detention basins would be included in a funding model as there based on their need to limit flows and prevent further pipe infrastructure upgrades. It would be fair for these to be aggregated as a whole of development requirement.

#### Stormwater Quality

Treatment of stormwater flows from developed areas is expected to be required in accordance with the pollutant reduction targets of 90% Gross Pollutants (GP), 80% Total Suspended Solids (TSS), 45% Total Phosphorus (TP) and 45% Total Nitrogen (TN) outlined in the *Tasmanian Stormwater Policy Guidance and Standards for Development* document.

Due to the significant portion of external undeveloped catchment flows and the nature of the development having the potential for staged or partial development of individual land parcels, it is expected to provide a better outcome to provide at source treatment/control. This would involve treatment at the localised discharges from the major developed areas before discharging into the existing gullys/watercourses, and/or





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by providing treatment at the time of development on each individual lot. By allowing the large portion of external catchment flows to discharge directly to Boyer Road, bypassing treatment, more efficient removal of pollutants from developed areas can be carried out with less dilution of polluted flows. This may assist in lowering the total sizes of treatment infrastructure required. Potential locations for combined treatment systems are indicated on the concept plans provided in Appendix A, however may be located elsewhere depending on final outlet locations.

It is expected that with the inclusion of the open space/watercourse areas that sufficient space will be available for treatment devices to meet the required pollutant reduction targets at the outlets from individual developed areas. The configuration of treatment devices may include both the use of 'natural' WSUD solutions (e.g. sediment ponds, swales, bioretention) and proprietary treatment devices (e.g. gross pollutant traps, filtration systems) with further assessment required to determine treatment sizes dependant on selected methods.

# Funding of Infrastructure

Local source control water quality devices to fall under local developer costs.

# Internal Road Bridges

While the internal road network built to LGAT standards and requirements will be the developers cost, there are two bridges crossing the water ways, one in 170 Boyer Road and the other in western Boyer Road property. Thes have been considered for contribution funding but on the basis that they are not required to facilitate the development for all six properties have been excluded from contribution funding.

# NBN

# Contact Person – Peter Freshney – Kisstel – 0417 287 006 Daniel Costa – NBN Co. daniel.costa@nbnco.com.au

Communication services in the form of the NBN are located at the intersection of Boyer Road and Serenity Drive. Both 50 and 170 Boyer Road are connected by Fixed Wireless to the NBN network. Contact has been made with a local designer and installer to receive an appropriate contact with Development Planning at NBN for discussion. Peter has advised that Daniel Costa of NBN Co. is our contact. We will follow up with Daniel in due course.

#### Update 6/12/24

Met with Daniel Costa from NBN. He is confident that NBN will supply the future subdivision and lead in infrastructure will be provided as part of an overall development plan. Costs will be part of developer charges similar to TasNetworks. Typically costs are \$1000 to \$1500 per lot.

#### Funding of Infrastructure

NBN infrastructure to fall under local developer costs.

# TasGas

TasGas have reticulation of Natural Gas on the eastern side of the Midland Highway in Bridgewater. The supply of Natural Gas is not considered to be an essential service but rather an optional service. Generally,





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TasGas will make a commercial decision in terms of supplying an area based on the level of interest of future customers.

#### Funding of Infrastructure

As this is not an essential service this will not form any part of the funding agreement.

# Department of State Growth - Entry Road Intersections

Advice has been received from Keith Midson concerning the treatment at the three entry road locations. A CHR(S) treatment is appropriate at all three locations. A CHR(S) treatment is approximately 160m long and involves introducing pavement widening, line marking and signage. A Bill of Quantities will be prepared for the Quantity Surveyor.

The construction of the CHR(S) intersections will facilitate development for the total land holding and the total cost for the three intersections will form part of the contributions plan.

Further to the intersections, a possible shared bike and pedestrian path from the Main Street to the entry point in 50 Boyer Road is being considered. Space and terrain may limit this proposal.

#### Funding of Infrastructure

The construction of three intersections to be included in a contributions plan and cost distributed on a per lot or developable land basis. A bike shared pathway would be included in a contributions scheme.

# Tas Irrigation

Inquiries with Tas Irrigation have taken place after the LIST showed an irrigation pipeline located in the property. This is a DN630 irrigation line that runs from the north at Cobbs Hill Road into 25 Cobbs Hill Road and then into 50 Boyer Road before leaving the property crossing Boyer Road, under the rail corridor and into the Derwent Estuary crossing the river to Granton. The proposed layout keeps this irrigation infrastructure in proposed public land areas.

# Conclusion

The Boyer Road development requires the provision of new services infrastructure to service approximately 360 residential lots. Early discussions with service authorities have been undertaken alongside preliminary concept design to determine the most likely and/or appropriate way forward for delivery of services.

Sewer infrastructure is expected to require the provision of three separate sewage pump stations at the low points of each catchment for pumping to existing gravity infrastructure outside 24 Boyer Road to the east. Upgrades to existing Nielsen Pde SPS will be required and include additional gravity and rising mains

Water supply will come from the Reservoir on Cobbs Hill Road. A minimum DN200 connection with a new main required to feed the development within a new road reserve or public reserve through the northern properties. To meet the supply needs a volume upgrade will be required to the reservoir.

To manage stormwater from both the site and existing catchments, stormwater detention is proposed within the two eastern-most catchments 1 & 2 with approximately 1,300m<sup>3</sup> and 2,360m<sup>3</sup> of detention volume expected to be required respectively. The western-most catchment 3 is not expected to require detention.





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All three catchments are to discharge through existing culverts beneath Boyer Road. Stormwater treatment is recommended to be provided at the source, either at the discharge from each developed area or on an individual lot basis to more effectively treat more highly polluted flows and minimise required infrastructure.

Power and communications supply is achievable with connection to existing infrastructure or augmented infrastructure and is not considered a risk to the development. TasNetworks will bundle supply and reticulation into a per lot charge passed onto the developer. NBN will take the same approach with guarantee to supply and cost recovery via a developer charge.

TasGas is considered an optional service with TasGas to determine if servicing the development is to be commercially viable.

The Traffic Engineer has provided intersection details and the cost of these will be determined and included in an overall contributions plan.

To assist the Quantity Surveyor a scope discussion is provided in Table format in Appendix E.

Yours Faithfully

Rodney Jesson Director Civil and Infrastructure





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Appendix A – Concept Site Services Plans





	CLIENT:	HOLMESDYER	TITLE: CONCEPT STORMWATER SERVICE PLAN
	PROJECT:	BOYER ROAD PRECINCT	
			SCALE: 1:2500 SHEET SIZE: A1 DWGs IN SET: -
.00	ADDRESS:	50 - 170 BUYER RUAD BRIDGEWATER	PROJECT No: 251013 DWG No: C501 REV: A



PVD 31-01-25

BY: DATE:

APPROVED: -

ACRED. No: -

DATE: -

Α	INFORMATION PURPOSES
EV:	ISSUED FOR / DESCRIPTION:

	CLIENT: HOLMESDYER	TITLE: CONCEPT SEWER + WATER SERVICE PLAN - SHEET 1
	PROJECT: BOYER ROAD PRECINCT	
•		SCALE: 1:2500 SHEET SIZE: A1 DWGs IN SET: -
<b>P.</b> 03 6388 9200	ADDRESS: 50 - 170 BUYER ROAD BRIDGEWATER	PROJECT NO: 251013 DWG NO: C502 REV: A

22–24 Paterson Street

Launceston TAS 7250



REV: ISSUED FOR / DESCRIPTION:

	CLIENT:	HOLMESDYER	TITLE: CONCEPT SEWER + WATER SERVICE PLAN - SHEET 2
	PROJECT:	BOYER ROAD PRECINCT	
			SCALE: 1:2500 SHEET SIZE: A1 DWGs IN SET: -
00	ADDRESS:	50 - 170 BUYER RUAD BRIDGEWATER	PROJECT No: 251013 DWG No: C503 REV: A



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Appendix B – Taswater Correspondence

Update 22/11/24 Attached



# Water and Sewerage Servicing Advice

TasWater Reference No.	TWSI 2024/00697-BTN
Date of response	22/11/2024 19/11/2024 05/11/2024
TasWater Contact	Elio Ross
Phone No.	0467 874 330
Response issued to	
Name	Rare Innovation Pty Ltd
Address	22-24 Paterson Street LAUNCESTON, TAS 7250
Contact details	approvals@rarein.com.au
Development details	
Address	170 BOYER RD, BRIDGEWATER
Property ID (PID)	7676396
Description of development	Proposed Subdivision 250-300 Lot

#### Advice

Hi Paul,

Thanks for the call earlier regarding the proposed 300 lots subdivision at 170 BOYER RD, BRIDGEWATER.

Pending the official comments back for our assets and modelling teams, I have reviewed the documentation to provide some preliminary comments.

It's assumed that the sewer is to fall to Riverside Drive Sewage Pumping Station BRIDGEWATER, (id GRPSP12). Likely several upgrades would be required, depending on the proposed discharge location it looks as though pipes will need to be upsized as well as SPS and storage locations. See updated advice below

Water, there were no concept plans provided with potential connection points nominated for the water network. Currently, the area is partly zoned in the Boyer Restricted Zone (BRW16), Pressure Head Level 161.7m, assuming this is because of its coming off the DN648 (id: A3352946) Bulk main and not suitable for a connection for this development.

The other part of the development is located at 50 BOYER RD, which is zoned as Bridgewater Reservoir Zone (BRW1), with a head of 91m. This zone is more suitable for pressures given the approximate elevation of the lots range is 10m to 50m.



It is unclear where the proposed connections to the water network to feed the development will be located. The nearest location would be D100 (id A199973), which is too small to feed this development.

Typically, a single-direction feed would require a DN200 connection, or at least 2 x dn150 connections on a well-looped stable network.

Possibly a min DN200 connection to DN375 (id A203814) at the Bridgewater Reservoir, looped down to Serenity Drives DN100 would be the best approach.

Please note that the new water main cannot be in private property, the upper section of the loop will need to be located in a new roadway or council reserve.

I have sketched this below in green.



Please note that this is an early, provisional review of the area. More detailed modelling and asset reviews are being conducted, which will result in changes to the current assessment. This process will take additional time to complete.



#### **Sewer Gravity:**

Modelling analysis indicated that in the absence of the development, there is already a huge strain on the system with an abundance of excessively surcharging sewers and significant downstream capacity issues.

To accommodate this development all of the gravity mains between the discharge of the Riverside Drive SPS rising main to the Nielsen Parade SPS will need upsizing. TasWater may consider in a joint venture with the developer. However, this will require further investigations, consideration and approvals that will need to be confirmed.

The developer may also have to contribute to the upsizing of gravity mains between the discharge of the Nielsen Parade SPS rising main and the STP.

TasWater cannot accept the risks of allowing this development to connect without any infrastructure upgrades occurring.

# SPS:

The development impacts two downstream SPS's GRPSP11 Nielsen Parade SPS and GRPSP12 Riverside Drive SPS.

#### **GRPSP11 Nielsen Parade SPS**

The downstream SPS "GRPSP11 Nielsen Parade SPS" currently has insufficient storage to support the proposed development. The total additional storage required at this site without the development is 22.5 kL. The total additional storage required at this site including the development is 52.6 kL. TasWater will hence be responsible for 22.5 kL and the developer responsible for 30.1 kL.

#### Pumps:

The pump station prior to the development fails to meet pump requirements by 10.3 l/s. With this development included, the pump station fails to meet pump requirements by 21.1 l/s.

#### **Rising Main:**

Following the required pump upgrade specified above, the rising main will need upsizing to accommodate the development.

#### **GRPSP12 Riverside Drive SPS**

The downstream SPS "GRPSP12 Riverside Drive SPS" currently has insufficient storage to support the proposed development. The total additional storage required at this site without the development is 0 kL. The total additional storage required at this site including the development is 25 kL. TasWater will hence be responsible for 0 kL and the developer responsible for 25 kL.

#### Pumps:

The pump station prior to the development meets pumping requirements. With this development included, the pump station fails to meet pump requirements by 0.8 l/s.

# **Rising Main:**

Following the required pump upgrade specified above, the rising main will need upsizing to accommodate the development.

#### STP:

The Green Point STP has sufficient hydraulic and treatment capacity to accommodate the additional loading from the proposed development.



#### Water Modelling:

Modelling indicates there is capacity (in regards to available pressures) in the existing network at 170 Boyer Rd, Bridgewater (pipe A199973 and A2O3814) to supply this proposed development without impacting adversely on the existing infrastructure or customers. Total boundary heads (HGL), not pressures, at the proposed connection point(s) with the reservoir set to  $1/3^{rd}$  full are:

Location	H.G.L. Peak hour	H.G.L Peak day + 10 l/s fire flow
A199973 (Boyer Rd)	86	86
A2O3814 (Rev outlet)	87	87

These are heads within the Taswater network, so they do not account for losses in customer piping and fittings. This result is based on a sound but imperfect knowledge of conditions on the field and those who use this information should allow an appropriate margin of error in their design.

We noticed that the site falls within the Boyer Restricted Zone, and we agree that connecting to the A3352946 bulk main is not suitable for this development. Therefore, we proposed a DN200 loop: one connection from the reservoir outlet and another from 50 Boyer Rd (see image below).

The reservoir is already fully committed prior to adding this development. If this development proceeds, it will require upsizing storage (ie new reservoir) by the devloper prior to the completion of the subdivion.





TasWater confirms that you have made a pre-lodgement enquiry for the above proposal. TasWater's servicing advice in this response to the above proposal is based on the water and sewerage components of the proposal only. The other aspects of the proposal will be assessed by the relevant Planning Authority, or the Development Assessment Panel established under section 60G of the *Land Use Planning and Approvals Act* ("the Act") where the proposal is declared as a major project under 60C of the Act.

Despite anything else in the servicing advice TasWater reserves its rights regarding this proposal, when it is submitted for assessment as required by law under the Act.

#### Fees

This assessment is provided at no cost. For details on fees applicable for a formal assessment please see <u>www.taswater.com.au</u>



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# Appendix C – Culvert Photos

Catchment 1 (50 Boyer Road)



Catchment 2 (170 Boyer Road)



Catchment 3 ('Boyer Road')







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Appendix D – TasNetworks Minutes



# Early Engagement Meeting - Subdivisions



Case Reference No.	CN24-316681		
Case Manager	Alan Heald		
Date	10/01/2025	Site Address	170 Boyer Road BRIDGEWATER
Attendees	Chris Symons, Alex Izbicki, Katherine Johnston, Alan Heald. Rodney Jesson (RodneyJ@rarein.com.au)		
Apologies	Stephen Holmes (customer) admin@holmesdyer.com.au Kirsty Spilsbury (consultant) approvals@rarein.com.au		

# Agenda

#	Items Discussed
1	Introductions
2	<ul> <li>Developer to discuss overview of Development:</li> <li>250 lots / multi staged development. Rodney has advised this could be up to 400 lots</li> <li>There are 6 parcels of land with 6 separate owners</li> </ul>
	<ul> <li>Planning is being undertaken by the local council and Rodney is acting as a planning consultant.</li> </ul>
	<ul> <li>Various stakeholders have attended a meeting to future plan and gain insight into the proposed development. Cost contribution is an important factor for the development planning.</li> </ul>
	• The planning approvals and stages have not been finalised and are subject to feasibility studies. Rodney is requesting this from TasNetworks. For TasNetworks to provide this, a staged plan will need to be provided.
	What outcomes does Developer require from this meeting:
	Is a Planning Permit in-place? □ Yes or ☑ No
	Note: application will not be progressed to design without receipt of the planning permit.
	Have any aboriginal /Enviro studies been undertaken as part of the planning permit? □ Yes or ☑ No Reports to be provided if available
	Have CAD files been supplied? □ Yes or ☑ No
	Is this Development Staged? ☑ Yes or □ No

	If yes staged, please provide a staging plan and confirm sequences of construction so TasNetworks can design a development plan.
	What are the intentions for street lighting, council will need to provide advice on what they require (rural subdivision)?
3	Customer choice options (note TasNetworks due to administrative requirements may not support changes to an option part way through the application process).
	Accredited Electrical Designers (AED) & Accredited Electrical Constructors (AEC)
	TasNetworks as Designers & Accredited Electrical Constructors (AEC)
	TasNetworks as Designers & TasNetworks as Constructor
4	What are the Developer's program timings?
	Note: Timing of TasNetworks Services
	Currently:
	<ul> <li>14 weeks to design from start date.</li> <li>20 weeks construction min</li> </ul>
	o 20 weeks construction min.
	36 weeks construction min. If a substation is required.
	Due to construction scheduling and procurement of equipment the construction stage may take approximately 20 weeks (indicative estimates only) to complete. In some instances, these timeframes may be extended due to the supply of materials, weather, the work schedule of construction crews, or delays that are attributable to you or your external service provider.
	This can push up-to <mark>24 weeks</mark> if a <mark>substation</mark> is involved.
	Do you require pre-purchase of substation if choosing TN to design/construct (18-20 weeks delivery time)?
5	Network planning feedback:
	<ul> <li>A staging plan is required for an accurate feasibility study to strategically place substations and HV network and provide a clear feasibility plan.</li> <li>The feeder that supplies development will need a network augmentation when development begins. The augmentation will be based on future needs.</li> </ul>
	<ul> <li>Network Augmentation is triggered by the required needs as the development proceeds, this trigger will try to be picked up at Stage 1 but may not occur until Stage 7 depending on what other infrastructure requirements are within the network.</li> </ul>

7	Design and Estimation Feedback:			
	<ul> <li>Estimation of the cost per lot is a rough order of magnitude and can be subject to various factors that can alter this amount and should only be a guided estimation only. A figure of approximately \$12,000.00 per lot was discussed however after receiving further technical advice at the conclusion of the meeting held, cost will be in the order of \$15,000.00 to \$20,000.00.</li> <li>Cost separation between network augmentation and the new network within the development is preferred for cost allocation.</li> </ul>			
	Note: TasNetworks conducts <mark>audits</mark> to assess the general compliance of the proposed (AED) designs. Acceptance by TasNetworks does not relieve the AED of responsibility for suitability or correctness of the design. The audit will be undertaken within ten business days, unless otherwise agreed.			
8	<ul> <li>Construction Auditing Requirement for Accredited Electrical Constructors (AEC).</li> <li>Open trench and cabling audit</li> <li>Pre-commissioning and As-built audit</li> </ul>			
	Note: A TasNetworks Electrical Safety Specialist will complete the above staged audits, and non-compliance will be communicated to the Constructor and the Developer. The Developer is to ensure Constructor rectifies the identified non-conformance to ensure the construction schedule goes ahead as planned to mitigate any further delays.			
	Note: regarding street and road names, can the developer please initiate this process early in the subdivision construction and provide to the AEC to ensure electrical labelling by the AEC is accurate at time of requesting the pre-commissioning and as-built audits Having labels fitted with roads 1,2 and 3 does not meet compliance and will not proceed to commissioning until such times as labelling is rectified.			
9	Are there any known risks or issues associated to this development?			
5				

	Actions	Due Date
1	Civil Site Contact #	
2	Staging plan from developer for feasibility study	





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# Appendix E – Funding Scope Table

Authority	Infrastructure Element	Contribution	Developer
		Funding	Cost
TasNetworks - Power			
	HV Augmentation and Site supply		Yes
	Internal Reticulation		Yes
Council - Stormwater			
	Detention Basins x 2 including Landscaping	Yes	
	Water Quality Devices – Source Control		Yes
	Local Stormwater Network		Yes
Council - Roads			
	Internal Road Network		Yes
	Internal Bridges x2		Yes
Council - Landscaping			
	Streetscape		Yes
	Parks and Waterways		Yes
	Walking Trails		Yes
	Playground	Yes	
Department of State			
Growth			
	Intersection Treatment	Yes	
	Shared Bike/Pedestrian Pathway	Yes	
TasWater - Water			
	New Reservoir Capacity Upgrade	Yes	
	DN200 and trunk watermain	Yes	
	Internal water reticulation		Yes
TasWater - Sewer			
	Sewer Pump Stations and Rising Main	Yes	
	Nielsen Pde Sewer Pump Station Upgrade	Yes (TW	
		contribution)	
	Downstream gravity main upgrades	Yes	
	Internal reticulation network		Yes
NBN			
	Infrastructure supply		Yes
	Internal reticulation		Yes





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Our Ref: 251013

8<sup>th</sup> November 2024

Update 19 November 2024

Update 31 January 2025

Holmes Dyer Pty Ltd Leve 3, Reid House, 15 Featherstone Place Adelaide SA 5000

#### **ATTENTION: S Holmes**

Dear Stephen

#### SITE ANALYSIS REPORT FOR INFRASTRUCTURE REQUIREMENTS - BOYER PSP

I am writing to you about the findings to date for the provision of infrastructure services to the proposed subdivision based on the Plan 1792-002 R2>14.01.25 provided by Holmes Dyer. This working document shows approximately 362 lots. The essential services required include power supply from TasNetworks, sewer systems and water supply from TasWater and communications from NBN Co. Brighton Council have provided feedback on stormwater management including stormwater quantity and quality control. Consideration has been given to the provision of natural gas supply from TasGas. Additionally, to the infrastructure requirements, information from Tas Irrigation is included showing the approximate location of an irrigation supply pipeline. Keith Midson Traffic Engineer, has provided intersection treatments for entry points along Boyer Road.

The collection of background information on the services will inform the establishment of funding models.

#### TasNetworks - Power Supply

#### Contact Person – Gary Hancl – 0438 338 060 Alan Heald and Alex Izbecki

Initial contact was made via email to Clodagh Doyle on 24/9/24. Due to some internal staff movements, setting up an Early Engagement meeting proved difficult. On 31/10/24 a discussion finally took place with Gary Hancl – Team Leader. This was a good opportunity to explain the project context and that we are planning the future and not the developers of the project. He understands that we are looking for high level design of the system that will lead to high level costings enabling a model for funding to be established that links the power supply infrastructure to the future development. He has now organised an Early Engagement meeting for 10am on 12/11/24 with the right people within TasNetworks. Further reporting to follow.

#### Update 19/11/24

Early Engagement meeting was held on 12/11/24 with Gary Hancl, Angela Trewin and Chris Symons. The Network Planning Representative was not at the meeting but was contacted after the meeting. The positive outcomes were that TasNetworks do not have any issues in terms of supply to this location. The design may





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include a looped supply especially if there is connectivity to Cobbs Hill Road. The internal requirements for supply are a 750kVa sub station per 100 lots. Preferable substation locations are central to the lots being supplied.

#### Update 10/1/24

After advice received from the Enquiry by Design Meeting from TasNetworks, the suggestion was to apply for a Feasibility Study. This occurred and on return to the office 6/1/25 and invitation for a new Early Engagement Meeting was received for 9/1/25 from different people in the Negotiated Connections Team, including Alan Heald and Katherine Johnstone. At the meeting Alex Izbecki was in attendance and understood that we are in a pre-planning and pre-subdivision phase.

The machinations of TasNetworks do not cater for pre-planning and they are used to dealing with subdivision developments with Planning Approval. They could do the feasibility study for a cost and would require a Staging Plan, which has now been produced.

Having said this they have indicated that they have an obligation to supply, that any costs associated with augmentation or upgrade requirements are bundled up into the developer costs that are normally associated with a subdivision. These developer costs include the HV lead in, sub-stations and underground reticulation. TasNetworks have indicated an indicative cost of \$15 000 to \$20 000 per lot.

Minutes of the meeting attached in Appendix D.

# Funding of Infrastructure Decision 31/1/25

Based on advice from TasNetworks the design and supply of the power to the future subdivision site is achievable. Augmentation of HV supply is achievable and final design will be developed with a development plan as per TasNetworks practice. The costs associated to achieve the required HV supply and the breakdown to LV via substations will be distributed to the developer generally as per lot basis. No costs are to be included in a contribution's arrangement.

# TasWater – Sewer Systems and Water Supply

# Contact Person - Elio Ross - 0467 874 330 Update provided 22/11/24

# Sewer System

In the first instance the advice from TasWater was to lodge a Service Enquiry. This was lodged with TasWater through their Portal on 16/10/24. To provide context to the servicing discussion we made calculations for Equivalent Tenements (ET's) for the sewer based on the three natural catchments within the development area. Each catchment has a low point adjacent to Boyer Road and will require a Sewer Pump Station. Pump Station 1 is located in 50 Boyer Road (Property ID: 7676361, Title Ref: 44724/8). Pump Station 2 is located in 170 Boyer Road (Property ID: 7676396, Title Ref: 44724/9) and Pump Station 3 is located furthest west in the property known as Boyer Road (Property ID: 1972194, Title Ref: 44724/2). The ET's for each catchment and Pump Station are listed in Table 1. A typical allowance for a 20% growth rate has also been included.





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The sewer pump stations have been sized at the highest level for environmental sensitivity based on their close proximity to the Derwent Estuary, meaning the storage time will be 8 hours (in accordance with *Taswater Supplement to WSA 04-2005 2.1 WSAA Sewage Pumping Code of Australia*) and hence the pump station size will be larger than a regular pump station with low environmental rating. The default environmental rating may be lowered subject to further risk assessments in consultation with Taswater, however due to the proximity to the Derwent Estuary, it is assumed that a high rating is appropriate and is likely to be required.

Consideration was given to a common rising main to allow each pump station to discharge into a common line before being discharged into a gravity manhole outside 24 Boyer Road. A professional discussion took place to investigate the possibility of achieving this outcome. After this discussion it was determined that it would not be possible to achieve this. There are two other possible ways to achieve the discharge requirements of the three pump stations. The first is to install an individual rising main to each sewer pump station. This means a common trench can be used from near Serenity Drive all the way to 24 Boyer Road. The other possible solution is to daisy chain the pump stations, that is one pumps to the other and then the next one. This means the pump stations would have to increase in size to accommodate the additional effluent discharged by the previous pump station and the inflow from the local catchment. A high-level cost analysis of these options will be carried out. In the first instance the multiple rising main option is likely to be the best option for cost but also for the fact that the stand-alone development can occur on the individual larger parcels. Please see new advice received about the pump stations and a common rising main.

The sewer pump station volumes for the multiple rising main option are shown in Table 1.

Pump Station ID	Developed Equivalent Tenements (ET's) (+20% Growth Rate)	Pump Station Volume	Pump Flow Rate
1 (50 Boyer Road)	134	20.1 m <sup>3</sup>	10.62 L/s
2 (170 Boyer Road)	248	37.2 m <sup>3</sup>	18.04 L/s
3 ('Boyer Road')	71	10.6 m <sup>3</sup>	5.95 L/s

Table 1 – Sewage Pump Station Parameters – Current Plan > 362 lots

Preliminary comment from Taswater was received on 05/11/24 (included in Appendix B), with more detailed advice from Taswater's assets and modelling teams still to follow. Taswater's preliminary comment assumed connection to the existing Riverside Drive Sewage Pumping Station (ID: GRPSP12) which represents another option for connection location, however indicated that several upgrades to the existing piped network and sewage pumping station would likely be required in this instance and the Riverside Drive SPS will not be included.

#### Service Enquiry Update 22/11/24

After discussion with TasWater and receiving the written advice indications are that a common rising main linking all three pumpstations is a possibility. This may change the configuration of each of the pump stations and will be subject of future design. The already calculated total capacity requirements for all three pump stations are unlikely to change and hence these sizes can be used to calculate costs in relation to a contribution model.





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TasWater have also provided additional information about their downstream sewerage infrastructure. They are indicating that there is significant strain on the gravity system, the existing sewer pump stations and rising mains. One positive is that the Green Point Sewerage Treatment Plant has adequate capacity to cater for the development.

Gravity main upgrades are required between the receiving manhole and the Nielsen Parade SPS. Further to this gravity and rising main upgrades maybe required from the Nielsen Parade SPS to the Green Point STP.

The Sewer Pump Station at Nielsen Parade requires an upgrade in volume, at total of 52.6kL. Taswater water have nominated that 30.1kL are the responsibility of the development. Further to this pump upgrades will be required with additional flow rate. The additional flow rate will require a larger rising main. These works are shown on Plan 251013 C503, with alternatives for the possible rising main upgrades. TasWater Servicing Advice found in Appendix B.

#### Funding of Infrastructure 31/1/25 - Sewer

The upgrade to the existing Nielsen Parade Pump Station will have shared costs between TasWater and an infrastructure contribution for Boyer PSP. Taswater have been clear in regard to volume upgrades and pump upgrades and their portion of these costs. The final part of the puzzle is the final rising main size. The cost difference is minimal in the rising main pipe diameter as most of the cost is in trenching and backfilling. A BoQ has been produced to assist the Quantity Surveyor to establish the apportioned costs to the contributions plan.

#### Water Supply

The creation of a reticulated water supply network to service the development will require connection to the existing Taswater supply network. The closest connection location is at the intersection of Serenity Drive and Boyer Road to an existing DN100 PVC main. Preliminary comment from Taswater (included in Appendix B) has indicated that the existing DN100 main will be too small to service the development with a DN200 connection (or 2 x DN150) typically required for a development of this size. The existing Taswater network within a reasonable proximity to the site (developed areas to the east) consists entirely of DN100 or less sized mains, meaning no suitable connection is in close proximity to the site from this area.

The existing developed area to the east is supplied from the single Bridgewater Reservoir (ID: HOBWS017), which is located adjacent to Cobbs Hill Road to the north of the site. Taswater has advised that connection for the site will likely be required directly from the existing DN375 main (ID: A203814) at the reservoir. Connection at this location would mean the new water supply will be required to run through areas of 29 Cobbs Hill Road not previously included for development. Taswater has also advised that the creation of a new road reserve would be required to house the new DN200 main as new water mains cannot be located within private property. It is likely that the new mains servicing the site will also require connection back into the existing network at Serenity Drive to create a closed loop.

No other existing infrastructure suitable for direct connection is present within the surrounding areas. Several large bulk transfer mains are located further north of the site as well as a DN630 irrigation main that partially runs through 50 Boyer Road, however these are not suitable nor available for connection. This means that Taswater's initial comments on connection location nearby the reservoir are very likely to be the required solution for the site.





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In preliminary assessment of possible connections to Serenity Drive, an EPANET model has been undertaken which has been adjusted to include supply from the Bridgewater Reservoir. Preliminary results from the model show that adequate pressures and flows are expected to be available to all lots on the site inclusive of both residential and fire flows to the most hydraulically disadvantaged lot. A screenshot of the EPANET model is provided in Figure 1 below.

#### Service Enquiry Update 22/11/24

#### Water Infrastructure

Previous advice in regards to supply from the reservoir on Cobbs Hill Road still applies including the trunk supply at DN200 then spreading to DN100 throughout the subdivision. The new advice is that the capacity of the reservoir is not adequate to maintain existing servicing and supply the proposed subdivision. To this end TasWater will require additional capacity, either by rebuilding the existing reservoir with additional capacity or by building a second reservoir to give the additional capacity. If there is adequate land area a second reservoir is the likely outcome as this will limit supply disruption. We will discuss further with TasWater about the like volume requirement for additional storage.



Figure 1 – EPANET Model (Cobbs Hill Road Connection)

This model provides DN 200 lead in from the reservoir spreading to a DN100 network within the subdivision.





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# Funding of Infrastructure 31/1/25 - Water

The requirement for additional reservoir storage at Cobbs Hill Road will fall under an infrastructure contribution model. Also included will be the DN200 connection from the Cobbs Hill Reservoir into the subdivision as it is required. Link water infrastructure that enables 50 Boyer Road, 170 Boyer Road and the third property on Boyer Road to be included in the infrastructure contribution model. A Bill of Quantities has been prepared for use by the Quantity Surveyor.

# Council - Stormwater Discharges and Onsite Detention - Internal Roads

# Contact Person - Leigh Wighton - 0418 569 044

A meeting with Brighton Council was undertaken on 05/11/24 which included discussion on stormwater quality and quantity management with the information provided below being in-line with these discussions and Council's direction/requirements.

The land has three overall catchments with each falling to Boyer Road as shown on the concept plans provided in Appendix A. Each catchment discharges via an existing pipe under Boyer Road. Catchment 1 (50 Boyer Road) has a twin DN375 discharging under Boyer Road into a table drain that runs towards the railway crossing point at Riverside Drive. Catchment 2 (170 Boyer Road) discharges via a DN900 under Boyer Road and then under the railway corridor and finally to the Derwent Estuary. Finally, Catchment 3 ('Boyer Road') has a DN1200 discharging under Boyer Road and then under the railway corridor to the Derwent Estuary. Photos of the existing pipes from the upstream side are provided in Appendix C.

The Catchments 2 & 3 have defined gullies, and because of the rural nature have farm dams capturing flows at various locations. While these dams provide online storage, this has been ignored for the overall catchment modelling. The catchment modelling was carried out using the software package DRAINS to determine the impact of increasing the impervious areas due to development. A screenshot of the DRAINS model is provided in Figure 2. The discharge parameters used consisted of maintaining the existing pipe sizes beneath Boyer Road to control flows up to the 1% AEP event including a climate change increase factor of 16.3%. The results of the modelling including likely detention sizes required, are shown in Table 2, noting that catchment 3 did not require any detention with the existing DN1200 pipe and roadside depression being adequate to build driving head and discharge of the post-development flows.

Catchment ID	Existing Pipe Discharge Size	Peak 1% AEP Post- Developed Flow (Generated by Catchment)	Approximate Detention Volume Required	Peak 1% AEP Post- Developed Flow Through Culvert (With Detention)
1 (50 Boyer Road Discharge)	2 x DN375	1.04 m³/s	1,300 m <sup>3</sup> (Approx. 1400m <sup>2</sup> , 2m max. depth)	0.358 m³/s
2 (170 Boyer Road Discharge)	DN900	3.57 m <sup>3</sup> /s	2,360 m <sup>3</sup> (Approx. 2000m <sup>2</sup> , 3m max. depth)	2.23 m³/s

Table 2 – Stormwater Flows and Detention Requirements





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3 ('Boyer Road'	DN1200	3.56 m <sup>3</sup> /s	Nil (sufficient pipe	3.35 m <sup>3</sup> /s
Discharge)			capacity/inlet	
			storage)	

With the presence of an existing gully within Catchment 2, the topography is expected to lend itself to the creation of a suitable detention basin. The topography adjacent to the outlet for Catchment 1 is less well defined in regards to an existing low point/water course and would likely require additional earthworks to construct a suitable detention basin.



Figure 2 – DRAINS Model (1% AEP Event)

#### Funding of Infrastructure

It is likely that the detention basins would be included in a funding model as there based on their need to limit flows and prevent further pipe infrastructure upgrades. It would be fair for these to be aggregated as a whole of development requirement.

#### Stormwater Quality

Treatment of stormwater flows from developed areas is expected to be required in accordance with the pollutant reduction targets of 90% Gross Pollutants (GP), 80% Total Suspended Solids (TSS), 45% Total Phosphorus (TP) and 45% Total Nitrogen (TN) outlined in the *Tasmanian Stormwater Policy Guidance and Standards for Development* document.

Due to the significant portion of external undeveloped catchment flows and the nature of the development having the potential for staged or partial development of individual land parcels, it is expected to provide a better outcome to provide at source treatment/control. This would involve treatment at the localised discharges from the major developed areas before discharging into the existing gullys/watercourses, and/or





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by providing treatment at the time of development on each individual lot. By allowing the large portion of external catchment flows to discharge directly to Boyer Road, bypassing treatment, more efficient removal of pollutants from developed areas can be carried out with less dilution of polluted flows. This may assist in lowering the total sizes of treatment infrastructure required. Potential locations for combined treatment systems are indicated on the concept plans provided in Appendix A, however may be located elsewhere depending on final outlet locations.

It is expected that with the inclusion of the open space/watercourse areas that sufficient space will be available for treatment devices to meet the required pollutant reduction targets at the outlets from individual developed areas. The configuration of treatment devices may include both the use of 'natural' WSUD solutions (e.g. sediment ponds, swales, bioretention) and proprietary treatment devices (e.g. gross pollutant traps, filtration systems) with further assessment required to determine treatment sizes dependant on selected methods.

#### Funding of Infrastructure

Local source control water quality devices to fall under local developer costs.

# Internal Road Bridges

While the internal road network built to LGAT standards and requirements will be the developers cost, there are two bridges crossing the water ways, one in 170 Boyer Road and the other in western Boyer Road property. Thes have been considered for contribution funding but on the basis that they are not required to facilitate the development for all six properties have been excluded from contribution funding.

# NBN

# Contact Person – Peter Freshney – Kisstel – 0417 287 006 Daniel Costa – NBN Co. daniel.costa@nbnco.com.au

Communication services in the form of the NBN are located at the intersection of Boyer Road and Serenity Drive. Both 50 and 170 Boyer Road are connected by Fixed Wireless to the NBN network. Contact has been made with a local designer and installer to receive an appropriate contact with Development Planning at NBN for discussion. Peter has advised that Daniel Costa of NBN Co. is our contact. We will follow up with Daniel in due course.

#### Update 6/12/24

Met with Daniel Costa from NBN. He is confident that NBN will supply the future subdivision and lead in infrastructure will be provided as part of an overall development plan. Costs will be part of developer charges similar to TasNetworks. Typically costs are \$1000 to \$1500 per lot.

#### Funding of Infrastructure

NBN infrastructure to fall under local developer costs.

# TasGas

TasGas have reticulation of Natural Gas on the eastern side of the Midland Highway in Bridgewater. The supply of Natural Gas is not considered to be an essential service but rather an optional service. Generally,





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TasGas will make a commercial decision in terms of supplying an area based on the level of interest of future customers.

#### Funding of Infrastructure

As this is not an essential service this will not form any part of the funding agreement.

# Department of State Growth - Entry Road Intersections

Advice has been received from Keith Midson concerning the treatment at the three entry road locations. A CHR(S) treatment is appropriate at all three locations. A CHR(S) treatment is approximately 160m long and involves introducing pavement widening, line marking and signage. A Bill of Quantities will be prepared for the Quantity Surveyor.

The construction of the CHR(S) intersections will facilitate development for the total land holding and the total cost for the three intersections will form part of the contributions plan.

Further to the intersections, a possible shared bike and pedestrian path from the Main Street to the entry point in 50 Boyer Road is being considered. Space and terrain may limit this proposal.

#### Funding of Infrastructure

The construction of three intersections to be included in a contributions plan and cost distributed on a per lot or developable land basis. A bike shared pathway would be included in a contributions scheme.

# Tas Irrigation

Inquiries with Tas Irrigation have taken place after the LIST showed an irrigation pipeline located in the property. This is a DN630 irrigation line that runs from the north at Cobbs Hill Road into 25 Cobbs Hill Road and then into 50 Boyer Road before leaving the property crossing Boyer Road, under the rail corridor and into the Derwent Estuary crossing the river to Granton. The proposed layout keeps this irrigation infrastructure in proposed public land areas.

# Conclusion

The Boyer Road development requires the provision of new services infrastructure to service approximately 360 residential lots. Early discussions with service authorities have been undertaken alongside preliminary concept design to determine the most likely and/or appropriate way forward for delivery of services.

Sewer infrastructure is expected to require the provision of three separate sewage pump stations at the low points of each catchment for pumping to existing gravity infrastructure outside 24 Boyer Road to the east. Upgrades to existing Nielsen Pde SPS will be required and include additional gravity and rising mains

Water supply will come from the Reservoir on Cobbs Hill Road. A minimum DN200 connection with a new main required to feed the development within a new road reserve or public reserve through the northern properties. To meet the supply needs a volume upgrade will be required to the reservoir.

To manage stormwater from both the site and existing catchments, stormwater detention is proposed within the two eastern-most catchments 1 & 2 with approximately 1,300m<sup>3</sup> and 2,360m<sup>3</sup> of detention volume expected to be required respectively. The western-most catchment 3 is not expected to require detention.




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All three catchments are to discharge through existing culverts beneath Boyer Road. Stormwater treatment is recommended to be provided at the source, either at the discharge from each developed area or on an individual lot basis to more effectively treat more highly polluted flows and minimise required infrastructure.

Power and communications supply is achievable with connection to existing infrastructure or augmented infrastructure and is not considered a risk to the development. TasNetworks will bundle supply and reticulation into a per lot charge passed onto the developer. NBN will take the same approach with guarantee to supply and cost recovery via a developer charge.

TasGas is considered an optional service with TasGas to determine if servicing the development is to be commercially viable.

The Traffic Engineer has provided intersection details and the cost of these will be determined and included in an overall contributions plan.

To assist the Quantity Surveyor a scope discussion is provided in Table format in Appendix E.

Yours Faithfully

Rodney Jesson Director Civil and Infrastructure





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Appendix A – Concept Site Services Plans





	CLIENT:	HOLMESDYER	TITLE: CONCEPT STORMWATER SERVICE PLAN
	PROJECT:	BOYER ROAD PRECINCT	
			SCALE: 1:2500 SHEET SIZE: A1 DWGs IN SET: -
.00	ADDRESS:	50 - 170 BUYER RUAD BRIDGEWATER	PROJECT No: 251013 DWG No: C501 REV: A



BY: DATE:

APPROVED: -

A INFORMATION PURPOSES REV: ISSUED FOR / DESCRIPTION:

	CLIENT:	HOLMESDYER	TITLE: CONCEPT SEWER + WATER SERVICE PLAN - SHEET 1
	PROJECT:	BOYER ROAD PRECINCT	
			SCALE: 1:2500 SHEET SIZE: A1 DWGs IN SET: -
00	ADDRESS:	50 - 170 BUYER RUAD BRIDGEWATER	PROJECT No: 251013 DWG No: C502 REV: A

Launceston TAS 7250

ACRED. No: -

DATE: -



REV: ISSUED FOR / DESCRIPTION:

	CLIENT:	HOLMESDYER	TITLE: CONCEPT SEWER + WATER SERVICE PLAN - SHEET 2
	PROJECT:	BOYER ROAD PRECINCT	
			SCALE: 1:2500 SHEET SIZE: A1 DWGs IN SET: -
00	ADDRESS:	50 - 170 BUYER RUAD BRIDGEWATER	PROJECT No: 251013 DWG No: C503 REV: A



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Appendix B – Taswater Correspondence

Update 22/11/24 Attached



# Water and Sewerage Servicing Advice

TasWater Reference No.	TWSI 2024/00697-BTN	
Date of response	22/11/2024 19/11/2024 05/11/2024	
TasWater Contact	Elio Ross	
Phone No.	0467 874 330	
Response issued to		
Name Rare Innovation Pty Ltd		
Address	22-24 Paterson Street LAUNCESTON, TAS 7250	
Contact details	approvals@rarein.com.au	
Development details		
Address	170 BOYER RD, BRIDGEWATER	
Property ID (PID) 7676396		
Description of development	Proposed Subdivision 250-300 Lot	

### Advice

Hi Paul,

Thanks for the call earlier regarding the proposed 300 lots subdivision at 170 BOYER RD, BRIDGEWATER.

Pending the official comments back for our assets and modelling teams, I have reviewed the documentation to provide some preliminary comments.

It's assumed that the sewer is to fall to Riverside Drive Sewage Pumping Station BRIDGEWATER, (id GRPSP12). Likely several upgrades would be required, depending on the proposed discharge location it looks as though pipes will need to be upsized as well as SPS and storage locations. See updated advice below

Water, there were no concept plans provided with potential connection points nominated for the water network. Currently, the area is partly zoned in the Boyer Restricted Zone (BRW16), Pressure Head Level 161.7m, assuming this is because of its coming off the DN648 (id: A3352946) Bulk main and not suitable for a connection for this development.

The other part of the development is located at 50 BOYER RD, which is zoned as Bridgewater Reservoir Zone (BRW1), with a head of 91m. This zone is more suitable for pressures given the approximate elevation of the lots range is 10m to 50m.



It is unclear where the proposed connections to the water network to feed the development will be located. The nearest location would be D100 (id A199973), which is too small to feed this development.

Typically, a single-direction feed would require a DN200 connection, or at least 2 x dn150 connections on a well-looped stable network.

Possibly a min DN200 connection to DN375 (id A203814) at the Bridgewater Reservoir, looped down to Serenity Drives DN100 would be the best approach.

Please note that the new water main cannot be in private property, the upper section of the loop will need to be located in a new roadway or council reserve.

I have sketched this below in green.



Please note that this is an early, provisional review of the area. More detailed modelling and asset reviews are being conducted, which will result in changes to the current assessment. This process will take additional time to complete.



## **Sewer Gravity:**

Modelling analysis indicated that in the absence of the development, there is already a huge strain on the system with an abundance of excessively surcharging sewers and significant downstream capacity issues.

To accommodate this development all of the gravity mains between the discharge of the Riverside Drive SPS rising main to the Nielsen Parade SPS will need upsizing. TasWater may consider in a joint venture with the developer. However, this will require further investigations, consideration and approvals that will need to be confirmed.

The developer may also have to contribute to the upsizing of gravity mains between the discharge of the Nielsen Parade SPS rising main and the STP.

TasWater cannot accept the risks of allowing this development to connect without any infrastructure upgrades occurring.

# SPS:

The development impacts two downstream SPS's GRPSP11 Nielsen Parade SPS and GRPSP12 Riverside Drive SPS.

### **GRPSP11 Nielsen Parade SPS**

The downstream SPS "GRPSP11 Nielsen Parade SPS" currently has insufficient storage to support the proposed development. The total additional storage required at this site without the development is 22.5 kL. The total additional storage required at this site including the development is 52.6 kL. TasWater will hence be responsible for 22.5 kL and the developer responsible for 30.1 kL.

#### Pumps:

The pump station prior to the development fails to meet pump requirements by 10.3 l/s. With this development included, the pump station fails to meet pump requirements by 21.1 l/s.

#### **Rising Main:**

Following the required pump upgrade specified above, the rising main will need upsizing to accommodate the development.

#### **GRPSP12 Riverside Drive SPS**

The downstream SPS "GRPSP12 Riverside Drive SPS" currently has insufficient storage to support the proposed development. The total additional storage required at this site without the development is 0 kL. The total additional storage required at this site including the development is 25 kL. TasWater will hence be responsible for 0 kL and the developer responsible for 25 kL.

### Pumps:

The pump station prior to the development meets pumping requirements. With this development included, the pump station fails to meet pump requirements by 0.8 l/s.

# **Rising Main:**

Following the required pump upgrade specified above, the rising main will need upsizing to accommodate the development.

### STP:

The Green Point STP has sufficient hydraulic and treatment capacity to accommodate the additional loading from the proposed development.



## Water Modelling:

Modelling indicates there is capacity (in regards to available pressures) in the existing network at 170 Boyer Rd, Bridgewater (pipe A199973 and A2O3814) to supply this proposed development without impacting adversely on the existing infrastructure or customers. Total boundary heads (HGL), not pressures, at the proposed connection point(s) with the reservoir set to 1/3<sup>rd</sup> full are:

Location	H.G.L. Peak hour	H.G.L Peak day + 10 l/s fire flow
A199973 (Boyer Rd)	86	86
A2O3814 (Rev outlet)	87	87

These are heads within the Taswater network, so they do not account for losses in customer piping and fittings. This result is based on a sound but imperfect knowledge of conditions on the field and those who use this information should allow an appropriate margin of error in their design.

We noticed that the site falls within the Boyer Restricted Zone, and we agree that connecting to the A3352946 bulk main is not suitable for this development. Therefore, we proposed a DN200 loop: one connection from the reservoir outlet and another from 50 Boyer Rd (see image below).

The reservoir is already fully committed prior to adding this development. If this development proceeds, it will require upsizing storage (ie new reservoir) by the devloper prior to the completion of the subdivion.





TasWater confirms that you have made a pre-lodgement enquiry for the above proposal. TasWater's servicing advice in this response to the above proposal is based on the water and sewerage components of the proposal only. The other aspects of the proposal will be assessed by the relevant Planning Authority, or the Development Assessment Panel established under section 60G of the *Land Use Planning and Approvals Act* ("the Act") where the proposal is declared as a major project under 60C of the Act.

Despite anything else in the servicing advice TasWater reserves its rights regarding this proposal, when it is submitted for assessment as required by law under the Act.

#### Fees

This assessment is provided at no cost. For details on fees applicable for a formal assessment please see <u>www.taswater.com.au</u>



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# Appendix C – Culvert Photos

Catchment 1 (50 Boyer Road)



Catchment 2 (170 Boyer Road)



Catchment 3 ('Boyer Road')







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Appendix D – TasNetworks Minutes



# Early Engagement Meeting - Subdivisions



Case Reference No.	CN24-316681		
Case Manager	Alan Heald		
Date	10/01/2025	Site Address	170 Boyer Road BRIDGEWATER
Attendees         Chris Symons, Alex Izbicki, Katherine Johnston, Alan H           Rodney Jesson (RodneyJ@rarein.com.au)		nnston, Alan Heald. u)	
ApologiesStephen Holmes (customer) admin@holmesdyer.com.au Kirsty Spilsbury (consultant) approvals@rarein.com.au		Imesdyer.com.au Prarein.com.au	

# Agenda

#	Items Discussed
1	Introductions
2	<ul> <li>Developer to discuss overview of Development:</li> <li>250 lots / multi staged development. Rodney has advised this could be up to 400 lots</li> <li>There are 6 parcels of land with 6 separate owners</li> </ul>
	<ul> <li>Planning is being undertaken by the local council and Rodney is acting as a planning consultant.</li> </ul>
	<ul> <li>Various stakeholders have attended a meeting to future plan and gain insight into the proposed development. Cost contribution is an important factor for the development planning.</li> </ul>
	• The planning approvals and stages have not been finalised and are subject to feasibility studies. Rodney is requesting this from TasNetworks. For TasNetworks to provide this, a staged plan will need to be provided.
	What outcomes does Developer require from this meeting:
	Is a Planning Permit in-place? □ Yes or ☑ No
	Note: application will not be progressed to design without receipt of the planning permit.
	Have any aboriginal /Enviro studies been undertaken as part of the planning permit? □ Yes or ☑ No Reports to be provided if available
	Have CAD files been supplied? □ Yes or ☑ No
	Is this Development Staged? ☑ Yes or □ No

	If yes staged, please provide a staging plan and confirm sequences of construction so TasNetworks can design a development plan.	
	What are the intentions for street lighting, council will need to provide advice on what they require (rural subdivision)?	
3	Customer choice options (note TasNetworks due to administrative requirements may not support changes to an option part way through the application process).	
	Accredited Electrical Designers (AED) & Accredited Electrical Constructors (AEC)	
	TasNetworks as Designers & Accredited Electrical Constructors (AEC)	
	TasNetworks as Designers & TasNetworks as Constructor	
4	What are the Developer's program timings?	
	Note: Timing of TasNetworks Services	
	Currently:	
	<ul> <li>14 weeks to design from start date.</li> <li>20 weeks construction min</li> </ul>	
	o 20 weeks construction min.	
	36 weeks construction min. If a substation is required.	
	Due to construction scheduling and procurement of equipment the construction stage may take approximately 20 weeks (indicative estimates only) to complete. In some instances, these timeframes may be extended due to the supply of materials, weather, the work schedule of construction crews, or delays that are attributable to you or your external service provider.	
	This can push up-to <mark>24 weeks</mark> if a <mark>substation</mark> is involved.	
	Do you require pre-purchase of substation if choosing TN to design/construct (18-20 weeks delivery time)?	
5	Network planning feedback:	
	<ul> <li>A staging plan is required for an accurate feasibility study to strategically place substations and HV network and provide a clear feasibility plan.</li> <li>The feeder that supplies development will need a network augmentation when development begins. The augmentation will be based on future needs.</li> </ul>	
	<ul> <li>Network Augmentation is triggered by the required needs as the development proceeds, this trigger will try to be picked up at Stage 1 but may not occur until Stage 7 depending on what other infrastructure requirements are within the network.</li> </ul>	

7	Design and Estimation Feedback:
	<ul> <li>Estimation of the cost per lot is a rough order of magnitude and can be subject to various factors that can alter this amount and should only be a guided estimation only. A figure of approximately \$12,000.00 per lot was discussed however after receiving further technical advice at the conclusion of the meeting held, cost will be in the order of \$15,000.00 to \$20,000.00.</li> <li>Cost separation between network augmentation and the new network within the development is preferred for cost allocation.</li> </ul>
	Note: TasNetworks conducts <mark>audits</mark> to assess the general compliance of the proposed (AED) designs. Acceptance by TasNetworks does not relieve the AED of responsibility for suitability or correctness of the design. The audit will be undertaken within ten business days, unless otherwise agreed.
8	<ul> <li>Construction Auditing Requirement for Accredited Electrical Constructors (AEC).</li> <li>Open trench and cabling audit</li> <li>Pre-commissioning and As-built audit</li> </ul>
	Note: A TasNetworks Electrical Safety Specialist will complete the above staged audits, and non-compliance will be communicated to the Constructor and the Developer. The Developer is to ensure Constructor rectifies the identified non-conformance to ensure the construction schedule goes ahead as planned to mitigate any further delays.
	Note: regarding street and road names, can the developer please initiate this process early in the subdivision construction and provide to the AEC to ensure electrical labelling by the AEC is accurate at time of requesting the pre-commissioning and as-built audits Having labels fitted with roads 1,2 and 3 does not meet compliance and will not proceed to commissioning until such times as labelling is rectified.
9	Are there any known risks or issues associated to this development?

	Actions	Due Date
1	Civil Site Contact #	
2	Staging plan from developer for feasibility study	





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# Appendix E – Funding Scope Table

Authority	Infrastructure Element	Contribution	Developer
		Funding	Cost
TasNetworks - Power			
	HV Augmentation and Site supply		Yes
	Internal Reticulation		Yes
Council - Stormwater			
	Detention Basins x 2 including Landscaping	Yes	
	Water Quality Devices – Source Control		Yes
	Local Stormwater Network		Yes
Council - Roads			
	Internal Road Network		Yes
	Internal Bridges x2		Yes
Council - Landscaping			
	Streetscape		Yes
	Parks and Waterways		Yes
	Walking Trails		Yes
	Playground	Yes	
Department of State			
Growth			
	Intersection Treatment	Yes	
	Shared Bike/Pedestrian Pathway	Yes	
TasWater - Water			
	New Reservoir Capacity Upgrade	Yes	
	DN200 and trunk watermain	Yes	
	Internal water reticulation		Yes
TasWater - Sewer			
	Sewer Pump Stations and Rising Main	Yes	
	Nielsen Pde Sewer Pump Station Upgrade	Yes (TW	
		contribution)	
	Downstream gravity main upgrades	Yes	
	Internal reticulation network		Yes
NBN			
	Infrastructure supply		Yes
	Internal reticulation		Yes



# Stormwater Analysis Report

Holmes Dyer Pty Ltd Boyer PSP Boyer Road, Bridgewater

Prepared for:Holmes Dyer Pty LtdProject No:251013Document No:251013-RPT-001Issue No:01Revision No:B



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# DOCUMENT CONTROL

Project	Holmes Dyer – Boyer PSP – Boyer Road, Bridgewater
Report Title	Stormwater Analysis Report
Project No	251013
Document ID	251013-RPT-001
File Path	R:\Projects\2025\251000 Civil\251013-Boyer Road Precicnt0Civil Services\4 Internal\STW Report
Client	Holmes Dyer Ptd Ltd

		Record of F	Report		
lssue	Reason	Revision	Date	Prepared By	Approved By
01	Client Issue	В	14/03/2025	JS	RJ

	Dist	ribution of Report	
Company	Name & Address	Contact	Copies
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# 1. INTRODUCTION

Rare Innovation Pty Ltd (Rare) have been engaged to undertake investigation into the provision of infrastructure services to the proposed subdivision based on the Plan 1792-002>14.01.25 provided by Holmes Dyer. This working document shows approximately 362 lots for the potential future subdivision. This report focuses on the stormwater related aspects of the development including existing and proposed infrastructure, stormwater quantity management and stormwater quality treatment. The objective of this report is to inform early engineering design, planning and future requirements for the potential subdivision.

# 2. EXISTING SITE

2.1. Location & Property Details

The proposed development is located across six existing land titles in Bridgewater, Tasmania:

- 50 Boyer Road, Property ID: 7676361, Title Reference: 44724/8
- 170 Boyer Road, Property ID: 7676396, Title Reference: 44724/9
- 182 Boyer Road, Property ID: 1972194, Title Reference: 44724/2
- 25 Cobbs Hill Road, Property ID: 2097504, Title Reference: 135574/2
- 29 Cobbs Hill Road, Property ID: 2097491, Title Reference: 135574/1
- 31 Cobbs Hill Road, Property ID: 2808927, Title Reference: 152364/2

The site is zoned as Future Urban across the extent of the proposed subdivision area. The northern portions of the Cobbs Hill Road properties remain zoned as Landscape Conservation, however are outside the extend of the proposed subdivision areas. The site is bounded by Boyer Road to the south, Cobbs Hill Road to the north, existing Rural Living zoned residential developments to the east and existing rural/agricultural properties to the west.

### 2.2. Land Description

The development site currently contains multiple rural residential properties across the six existing titles with accesses onto Boyer Road and Cobbs Hill Road. The areas designated for future development as part of this subdivision (as shown in Figure 1) generally consist of cleared rural farmland. Areas to the north of the subdivision area and partly within the north-western corner of the subdivision area generally consist of existing native bushland.

The existing topography varies across the whole development site but typically falls towards the south-west and Boyer Road with undulating falls from the north-westerly to the south-easterly direction forming three distinct minor valleys with several existing drainage dams present. Falls typically range from 5-10% towards the south-west across the lower half of the existing six titles where the subdivision is predominantly located. Falls are typically higher in the range of 10-25% within the upper existing bushland areas.







Figure 1 – Existing Site/Development Area

# 2.3. Existing Stormwater System & Catchments

Due to the nature of the existing land being largely rural/agricultural with existing bushland, the drainage across the site consists predominantly of naturally formed topographical drainage lines along minor valleys and gullies through the various properties towards the south-west and Boyer Road. Three distinct drainage paths are present which ultimately reach Boyer Road at the eastern end of no. 50 Boyer Road (Catchment 1, [east]), approximately in the centre of no. 170 Boyer Road (Catchment 2 [central]) and in the centre of no. 182 Boyer Road (Catchment 3 [west]). Multiple man-made farm dams are present within the central and western catchments. Appendix A provides an overview of the catchments and flows for the site which are discussed in further sections of this report. The catchments shown are the combined existing and developed catchments as they are expected to be altered in the developed areas of the site, although only marginally.

The outflow from each existing catchment discharges under Boyer Road through existing concrete culverts, which subsequently flow through the adjacent rail line and into the Derwent River. The western catchment contains a single DN1200 culvert in an existing gully, the central catchment contains a single DN900 culvert also located in an existing gully, and the eastern catchment contains a twin DN375 culvert located in a shallower open drain adjacent to the road. Photos of the existing culverts are provided in Appendix B.





# 3. PROPOSAL

# 3.1. Proposed Development

The proposed development is for the creation of a new subdivision area consisting of 362 lots and associated road and services infrastructure. The development is to feature three new road accesses from Boyer Road and proposes to retain the existing natural drainage corridors for use in stormwater management for the site. Figure 2 shows the master plan for the proposed extent of development and general lot layout.



Figure 2 – Development Master Plan (Source: Holmes Dyer)

# 3.2. Stormwater Objective

The overall objective for stormwater management across the development is to ensure that stormwater flows through the development area and as generated by the development are sufficiently managed with no/minimal impact to the existing environment. Stormwater quantity and quality modelling has been undertaken and is discussed further in the relevant sections of this report addressing the increased flows and pollutants likely to be generated by the proposed development and how they may be managed. This report provides high-level desktop assessments to inform the future design of the subdivision.

The intent for stormwater infrastructure for the site is to utilise the existing infrastructure wherever possible to meet the above objectives and determine any new required infrastructure or modification that may be required.





## 3.3. Stormwater Quantity Analysis

Modelling of the proposed development/existing site catchments has been undertaken using the Watercom DRAINS software package. Model input data has been obtained from the AR&R Data Hub for the development location utilising an initial loss/continuing loss model in accordance with AR&R guidelines. The following key model input parameters were used:

- Impervious Area Initial Loss: 1.0mm
- Impervious Area Continuing Loss: 0mm/hr
- Pervious Area Initial Loss: 26.0mm
- Pervious Area Continuing Loss: 4.3mm/hr
- Climate Change loading factor of 16.3% rainfall intensity increase adopted.

The modelling that has been undertaken has utilised the existing culverts beneath Boyer Road effectively as the limiting discharge for each catchment. The intent is to ensure that if fully utilised, that the existing culverts do not overtop the road in storm events up to the 1% AEP (1 in 100 year) frequency. Any stormwater detention infrastructure that may be required to meet this objective has also been modelled with results from each of the three catchments provided in Table 1 below. A screenshot of the DRAINS model is also shown in Figure 3.

Additional consideration of recent climate change factor increases and worst-case scenarios for limiting flows to pre-development levels are also discussed further in Section 3.4.

Catchment ID	Existing Pipe Discharge Size	Peak 1% AEP Post- Developed Flow (Generated by Catchment)	Approximate Detention Volume Required	Peak 1% AEP Post- Developed Flow Through Culvert (With Detention)
1 (50 Boyer Road Discharge) (East)	2 x DN375	1.04 m <sup>3</sup> /s	1,300 m <sup>3</sup> (Approx. 1400m <sup>2</sup> , 2m max. depth)	0.358 m <sup>3</sup> /s
2 (170 Boyer Road Discharge) (Central)	DN900	3.57 m <sup>3</sup> /s	2,360 m <sup>3</sup> (Approx. 2000m <sup>2</sup> , 3m max. depth)	2.23 m <sup>3</sup> /s
3 (182 Boyer Road Discharge) (West)	DN1200	3.56 m <sup>3</sup> /s	Nil (sufficient pipe capacity/inlet storage)	3.35 m <sup>3</sup> /s

Table 1 – Stormwater Flows and Detention Requirements









The results shown in Table 1 for the 'Peak 1% AEP Post-Developed Flow (Generated by Catchment)', indicate the total flow that is expected to be generated by each catchment including the proposed developed areas and existing upstream properties/bushland areas. These flows were modelled as entering each respective culvert and detention storages were applied if required to ensure that the flows would not overtop Boyer Road. The existing gully depressions around the western and central culverts assist in allowing water to pond at the culvert inlet and build driving head in large flows, however the eastern-most culvert is situated in a shallower drain and does not allow for much ponding around the inlet in it's current state. It is expected that some minor works may be required around this culvert inlet to improve inlet capture efficiency.

From the modelling undertaken and as summarised in Table 1, Catchments 1 and 2 (east and central) will require detention storages to limit flows and allow discharge without overtopping through the existing culverts. A detention storage volume of approximately 1,300m<sup>3</sup> is expected to be required for Catchment 1 and 2,360m<sup>3</sup> for Catchment 2. With these detention storages in place the 'Peak 1% AEP Post-Developed Flow Through Culvert (With Detention)' is shown to be reduced, with these flows able to be adequately drained by the existing culverts. Modelling has indicated that the culvert for Catchment 3 is able to adequately discharge the 1% AEP developed flows without requiring additional detention storages.

The above detention storages were modelled utilising the existing valley depressions/topography to form an above ground basin. The detention basins may alter in size, shape and/or configuration when undertaken during more detailed design and as such the final flows discharged may change. However, with the detention basins modelled, this indicates that basins of sufficient size to achieve the required flow reductions are achievable. It is noted that the topography of Catchment 2 lends itself to the creation of a suitable detention basin, however Catchment 1 has a less defined depression/drainage route and will likely require additional earthworks.

The current basins have been modelled with a single control outlet only to ensure maximum flows for the 1% AEP event (climate change inclusive) are controlled. Additional multi-stage outlet control can be implemented during detailed design if required to control more frequent rainfall events.





It is expected that the approach of utilising the existing culverts and constructing detention basins within the site will be the most efficient approach to managing stormwater from the development rather than upgrading the existing culverts to allow for the full development's flows. Upgrades to the existing culverts would require significant works beneath Boyer Road (being a State Road owned by Department of State Growth) as well as additional upgrade works further downstream until reaching the Derwent River including potential upgrades being required to any drainage culverts beneath the adjacent rail line.

# 3.4. Stormwater Quantity Analysis - Climate Change Increases & Further Discharge Limiting

Further considerations for increased climate change loading and limiting of discharges to pre-development levels has also been assessed to provide guidance on the likely required detention infrastructure required in such scenarios.

Previous modelling as described in Section 3.3 used the capacity of the existing culverts beneath Boyer Road as the limiting factor for site discharges. It is noted that there may be further limiting factors downstream of these culvert locations (e.g. additional culverts or drainage infrastructure through the rail corridor) and so it may be appropriate to restrict flows further to pre-development levels should this be the case. As such, the stormwater runoff quantities generated by the site during a 1% AEP event (current condition, no climate change loading) have also been assessed and are presented in Table 2.

Recent revisions to AR&R have adjusted the climate change factors to be dependent on the storm durations being assessed, typically with increases to climate change loading multipliers across the board compared to previous values and in particular for short duration storms (increases of up to 77% to base rainfall intensities for SSP5-8.5 2090). Additional modelling has been undertaken in accordance with AR&R guidance from Watercom for modelling in DRAINS software. The following key model input parameters include adjustment factors also now provided as part of the revised AR&R data.

- Impervious Area Initial Loss: 1.0mm
- Impervious Area Continuing Loss: 0mm/hr
- Pervious Area Initial Loss: 30.42mm
- Pervious Area Continuing Loss: 5.977mm/hr
- Rainfall intensity data adjusted based on SSP5-8.5 Pathway (2090)

Both the adjusted climate change factors and the pre-development flows (as limits to site discharges) have been used to determine the required stormwater detention infrastructure in such a case, with summaries of flows provided in Table 2.





Catchment ID	1% AEP Pre- Development Flow	Peak 1% AEP Post- Developed Flow	Peak 1% AEP Post- Developed Flow (with Detention)	Approximate Detention Volume Required
1 (50 Boyer Road Discharge) (East)	0.692 m <sup>3</sup> /s	2.14 m³/s	0.343 m <sup>3</sup> /s	3,400 m <sup>3</sup> (Approx. 2000m <sup>2</sup> , 3m max. depth)
2 (170 Boyer Road Discharge) (Central)	2.51 m <sup>3</sup> /s	7.07 m <sup>3</sup> /s	2.44 m <sup>3</sup> /s	8,300 m <sup>3</sup> (Approx. 3,500m <sup>2</sup> , 5m max. depth)
3 (182 Boyer Road Discharge) (West)	2.46 m <sup>3</sup> /s	6.72 m <sup>3</sup> /s	2.47 m <sup>3</sup> /s	8,400 m <sup>3</sup> (Approx. 4,000m <sup>2</sup> , 3.5m max. depth)

 Table 2 – Pre-Development & Post-Development Flows (new climate change factors included)

The increase in stormwater runoff generated by each catchment is shown to be significant in comparison the those shown in Table 1 using the previous climate change factors with increases of 88-102%.

Catchment 1 (East) is likely to be able to accommodate the increased detention basin size required. It is noted that the capacity of the existing twin culvert in this location is actually less than the peak predevelopment 1% AEP flow and so the capacity of the culvert remains the limiting factor for discharge in this location. It is expected that the increased basin size noted in Table 2 will be able to limit flows to the lesser culvert capacity (and thus the pre-development flow also).

Catchment 2 (Central) is likely to be able to accommodate the increased detention basin size required in the same location as noted in Appendix A, however the shape and configuration of this basin is likely to be more constrained by the width of the open space area available. It is likely that a basin of this size will be approaching the limits of what is able to fit within this area, with the possibility of additional earthworks required to chase the basin out further uphill. There is also the possibility of splitting the detention into two separate basins with a second located further uphill, possibly at the location of one of the existing farm dams if required. It is noted than the pre-development flow for this catchment is approximately equal to the existing culvert capacity.

Catchment 3 (West) will require detention to accommodate this scenario where previously none was required for the scenario described in Section 3.3. The basin would likely be constructed in a similar position to the other two catchments at the low point of the open space area where the lowest existing farm dam is currently located. However, it is likely to encounter space constraints if constructing a single full size basin to the size noted in Table 2. Such a basin will be achievable and will meet the required targets, however will likely occupy the entire lower portion of the open space area up to approximately the 10m contour line. This is expected to impact on the space available for localised treatment locations. It is recommended that if this climate change/discharge scenario is adopted that the detention be split across two separate basins with one located in the same position as shown in Appendix A and another positioned uphill at the location of the existing farm dam in the northern corner of the developed area. The sizing of these basins to meet the pre-development discharge requirement will be approximately 7,000m<sup>3</sup> (3,700m<sup>2</sup>, 3.5m max. depth) for the lower basin and approximately 2,000m<sup>3</sup> (1,900m<sup>2</sup>, 3m max. depth) for the upper basin, providing some reduction in size of the lower basin from a single basin configuration.





By considering only the increase in climate change factors and continuing to utilise the maximum capacity of the existing culverts beneath Boyer Road, some reduction in the size of the western basin can be achieved due to spare culvert capacity available above pre-development levels. It is noted that the pre-development flows are approximately equal to the culvert capacity for the central catchment and are higher than the culvert capacity for the eastern catchment, so no further reductions to detention can be made in this scenario for these catchments. The resulting basin size that would be required for the western catchment in this scenario is approximately 5,100m<sup>3</sup> (3,300m<sup>2</sup>, 2.65m max. depth) and is likely to be achievable within the lowest portion of the open space area.

### 3.5. Stormwater Retention

With the creation of multiple detention basins, there is the potential for allowing for a permanent retention storage in each to create a permanent water body to improve natural values and/or for water re-use. The creation of retention volumes within each basin will require additional earthworks to increase the size of the basin, allowing for a permanent storage volume as well as sufficient additional capacity to allow for temporary detention in large rainfall events.

The size of the permanent storage area will be largely determined based on nominated landscape design, cost constraints and physical area limitations, however to ensure the retention storage is able to retain a permanent water body it is necessary to conduct a water balance check to ensure that evapotranspiration (ET) losses do not outweigh yearly rainfall inflows.

A maximum retention surface area has been estimated by modelling using MUSICX software using a 10year representative period of rainfall data. Net yearly inflow to the smallest catchment (eastern catchment) is approximately 32.5 ML/year.

The maximum retention pond surface area resulting in ET losses greater than the above net inflow is approximately 41,000m<sup>2</sup>.

With the areas available in the open space locations for the proposed development and considering existing topography, it is not expected that maximum sized ponds in excess of 2,000-3,000m<sup>2</sup> will be achievable. It is thus also expected that any permanent retention ponds are able to maintain yearly storage.

Due to no detention basin being required for the western catchment, it is expected that construction of a permanent retention pond would be achievable in this area.

Due to the size of detention basin for the central catchment (approximately 2000m<sup>2</sup>) requiring use of a large portion of the available open space area, it is not expected that this basin will also be able to be utilised as a permanent retention pond with limited additional space available. To provide a permanent retention pond, a second separate pond may be located further upstream of the detention basin.

It is possible that the detention basin for the eastern catchment may be increased in size to accommodate some permanent retention with sufficient open space available. However, the topography in this area does not feature a prominent low valley/drainage channel and more significant earthworks than the other two catchments would likely be required to cut and retain a pond/basin of sufficient size.

Re-use of water stored in permanent retention ponds is achievable, however is expected to be mostly dependent on available space and budget for construction of suitably sized storages to meet any re-use





volumes required. Catchment inflows are expected to be sufficiently high to assist in servicing any intended re-use, however will be dependent on the storage volumes constructed and frequency of draw-down from the storages.

## 3.6. Stormwater Quality Analysis

Treatment of stormwater flows is expected to be required in accordance with the pollutant reduction targets of 90% Gross Pollutants (GP), 80% Total Suspended Solids (TSS), 45% Total Phosphorus (TP) and 45% Total Nitrogen (TN) outlined in the *Tasmanian Stormwater Policy Guidance and Standards for Development* document.

The required treatment may be achieved in a number of ways by using proprietary treatment devices and/or natural treatment systems in multiple configurations. These include a combination of the below main configurations:

- Primary treatment in-line with catchment drainage courses, utilising sedimentation ponds or extended period detention.
- Localised treatment from individual development areas.
- Treatment provided on an individual lot basis.

With the site proposing to maintain the existing drainage routes for each catchment through the developed areas, an option for treatment may include utilising retention ponds, possibly in combination with the detention storages to achieve some of this treatment requirement. However, due to the large portion of external natural catchment flows that will continue to flow through the drainage course, this is not expected to be the most efficient treatment method. Treatment systems typically provide their most effective treatment when pollution concentrations in the inflows are high. Pollutant loading from non-developed areas such as rural catchments or native bushlands are typically low, whereas pollutant loading in runoff from developed urban catchments is typically higher. By attempting to treat both the existing natural catchment as well as the developed catchment, pollutant loadings will be diluted, resulting in less efficient treatment. Due to the larger flows from the combined catchment, the treatment systems would also likely be required to be larger in size to cope with the increased diluted flows. Additional tertiary treatment systems to remove finer particles and reduce Nitrogen levels would likely also be required. As such, this method of providing primary treatment within the drainage course is not recommended.

Alternative options which would provide a more efficient treatment system include at source treatment or localised treatment from each developed area. Individual areas of development will require localised discharges into the central drainage routes depending on site topography and available grades. Treatment may be provided at each discharge point into the drainage courses which will provide more efficient treatment of inflows having a higher pollutant concentration. This may be in the form of proprietary treatment devices at the outflow or in combination with natural solutions such as bioretention if available space allows. Additional treatment devices could also be incorporated into the streetscape such as biofiltration pits to assist as at-source control. Appendix A indicates several areas where localised treatment system. Appendix A also highlights the nominated treatment areas where it is likely that bio-retention may be more suitable, with urban catchment areas of sufficient size to warrant the additional maintenance from





construction of such bio-retention. These may however be dependent on impact from the chosen detention basin sizes/configurations also.

Treatment may also be provided on an individual lot basis by requiring the owner/developer of each future lot to provide treatment systems to meet the required pollutant reduction targets for their individual development. This ensures that a significant portion of the overall developed catchment is treated at the required level prior to discharge into the public system. However, this may place an unnecessary burden on individual residential owners/builders with upfront and maintenance costs that may be more effectively used to provide combined treatments at the point of discharge, resulting in an overall lesser sized system.

It is recommended that a combination of localised treatment to treat flows from sections of development areas be provided along with treatment on an individual lot basis for any larger individual lot developments (e.g. unit developments resulting in higher impervious surfaces and pollutant loads than a typical residential lot). The proposed development has been modelled using MUSICX software to analyse if this treatment method is achievable with the model configurations and results shown in Figures 4-6 and Tables 3-9 below.



Figure 4 – MUSICX Model – West Catchment



Figure 5 – MUSICX Model – Central Catchment







Figure 6 – MUSICX Model – East Catchment

Table 3 – MUSICX Model Results – West Catchment – Urban Area 01

	(-)			
	Sources	Residual Load	% Reduction	
Flow (ML/yr)	5.392	5.392	0	
Total Suspended Solids (kg/yr)	1071	82.55	92.29	
Total Phosphorus (kg/yr)	2.224	0.3269	85.3	
Total Nitrogen (kg/yr)	15.51	7.967	48.64	
Gross Pollutants (kg/yr)	187.1	0.4618	99.75	

# (1) West Catchment : Treatment Train Effectiveness : Local Area 01

 Table 4 – MUSICX Model Results – West Catchment – Urban Area 02

		(1) West (	Catchment :	Treatment T	rain Effectiveness : Local Area 02
		Sources	Residual Load	% Reduction	
Flow (	ML/yr)	41.89	41.89	0	
Total S	Suspended Solids (kg/yr)	8419	1065	87.35	
Total F	hosphorus (kg/yr)	17.22	3.313	80.77	
Total I	Nitrogen (kg/yr)	119.7	64.6	46.01	
Gross	Pollutants (kg/yr)	1453	59.71	95.89	





# Table 5 – MUSICX Model Results – Central Catchment – Urban Area 03

	(2) Central	cuterintent .	meannent	Hum Encenveness . Eocul / freu os
	Sources	Residual Load	% Reduction	
Flow (ML/yr)	10.28	10.28	0	
Total Suspended Solids (kg/yr)	2022	171.6	91.52	
Total Phosphorus (kg/yr)	4.259	0.6671	84.34	
Total Nitrogen (kg/yr)	29.6	15.34	48.17	
Gross Pollutants (kg/yr)	356.7	3.56	99	

# (2) Central Catchment : Treatment Train Effectiveness : Local Area 03

Table 6 – MUSICX Model Results – Central Catchment – Urban Area 04

(2) Central Catchment :	Treatment	Train Effectiveness :	Local Area 04
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	Sources	Residual Load	% Reduction
Flow (ML/yr)	31.97	31.97	0
Total Suspended Solids (kg/yr)	5090	615.2	87.91
Total Phosphorus (kg/yr)	9.929	1.819	81.68
Total Nitrogen (kg/yr)	73.54	39.33	46.51
Gross Pollutants (kg/yr)	787.3	16.24	97.94

 Table 7 – MUSICX Model Results – Central Catchment – Urban Area 05

	(2) Central	Catchment :	Treatment	Train Effectiveness : Local Area 05
	Sources	Residual Load	% Reduction	
Flow (ML/yr)	35.69	35.69	0	
Total Suspended Solids (kg/yr)	7174	834.3	88.37	
Total Phosphorus (kg/yr)	14.64	2.676	81.72	
Total Nitrogen (kg/yr)	102	54.22	46.85	
Gross Pollutants (kg/yr)	1238	37.26	96.99	

Table 8 – MUSICX Model Results – Central Catchment – Urban Area 06





# (2) Central Catchment : Treatment Train Effectiveness : Local Area 06

	Sources	Residual Load	% Reduction
Flow (ML/yr)	10.67	10.67	0
Total Suspended Solids (kg/yr)	2137	190.7	91.08
Total Phosphorus (kg/yr)	4.335	0.6703	84.54
Total Nitrogen (kg/yr)	30.39	15.72	48.26
Gross Pollutants (kg/yr)	370	3.971	98.93

Table 9 – MUSICX Model Results – East Catchment – Urban Area 07

Latest Run :	Treatment	Train	Effectiveness	: Local	Area 07
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	Sources	Residual Load	% Reduction
Flow (ML/yr)	27.38	27.38	0
Total Suspended Solids (kg/yr)	5442	525.8	90.34
Total Phosphorus (kg/yr)	11.1	1.835	83.48
Total Nitrogen (kg/yr)	78.22	41.08	47.48
Gross Pollutants (kg/yr)	950.1	17.88	98.12

The purpose of this modelling is to determine if a reasonable method of stormwater treatment is achievable for the development. Treatment of the final subdivision design may take many forms, with many options available for both proprietary and natural treatment systems. Modelling has been undertaken using a single proprietary hydrodynamic separator GPT (Atlan Vortceptor – offline configuration) at multiple assumed localised urban catchment outlets as a basic design solution.

From the results shown in Tables 3-9, each urban catchment area can be seen to achieve the required pollutant reduction targets of 90% Gross Pollutants (GP), 80% Total Suspended Solids (TSS), 45% Total Phosphorus (TP) and 45% Total Nitrogen (TN).





# 4. SUMMARY

The Boyer Road development will require the provision of additional stormwater infrastructure to ensure that any increases to flow quantities and pollutant loads are adequately managed and that impacts to the receiving environment are mitigated.

Expected stormwater runoff for the proposed development has been modelled using DRAINS software. It is proposed to utilise the existing stormwater culverts beneath Boyer Road as the limiting factor for stormwater quantity. To ensure that the capacity of the existing culverts for the 1% AEP storm event (16.3% climate change loading inclusive) is not exceeded, stormwater detention infrastructure will be required for two of the three catchments across the site (central and eastern catchments) with approximate volumes of 2,360m<sup>3</sup> and 1,300m<sup>3</sup> required respectively. The western culvert has been assessed to provide sufficient capacity for the developed flows and will not require stormwater detention.

Recent increases to climate change loading factors as well as limiting flows to pre-development levels has also been considered to provide advice on increased infrastructure requirements for this worst-case condition.

Stormwater treatment has been modelled using MUSICX software to determine if a basic treatment train option can achieve the required pollutant reduction targets for the development. The most appropriate and efficient method for treatment is expected to be by utilising treatment structures (proprietary or natural) at several localised discharges from areas of the urban development prior to discharge into the existing water courses flowing through the site. This allows natural flows from the upper catchment to continue to flow through the site unchanged, whilst the new urban areas are treated more effectively at higher pollutant concentrations. Results from modelling have indicated that treatment using the above method is achievable.

Permanent stormwater retention is also expected to be achievable from a water balance perspective, however will be largely dependent on available space and cost constraints.

Yours Faithfully

Rodney Jesson Director Civil and Infrastructure





# 5. APPENDIX A – CONCEPT STORMWATER SERVICE PLAN




	CLIENT: HOLMESDYER	TITLE: CONCEPT STORMWATER SERVICE PLAN
	PROJECT: BOYER ROAD PRECINCT	
		SCALE: 1:2500 SHEET SIZE: A1 DWGs IN SET: -
<b>)</b>	BRIDGEWATER	PROJECT No: <b>251013</b> DWG No: <b>C501</b> REV: <b>B</b>

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## 6. APPENDIX B – CULVERT PHOTOS

Eastern Catchment 1 (50 Boyer Road)



Central Catchment 2 (170 Boyer Road)



Western Catchment 3 (182 Boyer Road)







22-24 Paterson Street Launceston, TAS 7250

P. 6388 9200

rarein.com.au

Our Ref: 251013

4<sup>th</sup> June 2025

Holmes Dyer Level 3, Reid House, 15 Featherstone Place Adelaide SA 5000

### ATTENTION: N TAYLOR

Dear Nitsan,

This letter is provided as confirmation that the roads as detailed in the provided concept documentation (specifically Rare concept plans 251013-C – Road 25-05-21) comply with the relevant standards as listed below and with the following additional notes:

- Tasmanian Municipal Standard Drawings v3 December 2020 ('LGAT Standard Drawings')
- Tasmanian Subdivision Guidelines October 2013
- Austroads Guide to Road Design Part 3: Geometric Design Edition 3.4
- AS 2890.5 Parking Facilities, Part 5: On-Street Parking
- AS/NZS 2890.1 Parking Facilities, Part 1: Off-Street Car Parking

The Tasmanian Subdivision Guidelines and the LGAT Standard Drawings have predominantly informed the concept designs. The main point of note in relation to compliance with these standards/guidelines is with vertical grading due to the existing site grades being moderate to steep in some areas. The concept drawings noted above provide preliminary longitudinal sections for the proposed roads. The Tasmanian Subdivision Guidelines Clause 5.8, 11.4.1 (ii) & (iii) specify that the longitudinal road (and kerb) gradient shall be between 0.5% to 14%, except that in special circumstances the council may permit a grade of 20% for short lengths of road up to 70m. As shown in the above noted drawings, all grades are able to be designed within this range, except for Road 8 for which grades may exceed 14% (up to 16.19%) between chainages 230m – 289.575m (60m approx.). However this still complies with the above condition allowing for approval of such grades below 20% for lengths  $\leq$ 70m by Council, for which it is potentially warranted in this case due to existing grades.

Further refinement of road levels can be undertaken to increase earthworks cut/fill to minimise these grades if required, at the expense of additional cost of construction. Further refinement of road reserve cross section geometry may also be undertaken to optimise the location/offset of footpaths closer to kerbs in places where reduction of cut/fill earthworks may be warranted (e.g. limit batter encroachment into lots by moving footpath hard against back of kerb to reduce overall road footprint).





22-24 Paterson Street Launceston, TAS 7250

**P.** 6388 9200

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The current typical cross section designs specify nature strip batter transitions to allow for driveways to be constructed with appropriate grade changes in accordance with the LGAT Standard Drawings and AS/NZS 2890.1.

If you require anything further please contact us.

Yours faithfully,

hoh Rodney Jesson

Director - Civil





INFORMATION PURPOSES
ISSUED FOR / DESCRIPTION:

	STATUS: PRELIMINARY / INFORMATION	DESIGN BY: RJ DESIGN CHK: JS	rare	CLIENT: HOLMESDYER	TITLE: CIVIL WORKS PLAN
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BY: DATE:	APPROVED: - ACRED. No: -	DATE: -	Launceston TAS 7250 <b>P.</b> 03 6388 920	BRIDGEWATER	PROJECT No: 251013 DWG No: C400 REV: A





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Existing Levels	17.687 17.687 18.087	18.378 18.614	19.057 19.057 19.654 20.081	20.081	21.410	23.252	24.606 25 724	25.124	27.021 27.349 27.938	28.283 28.381 29.104	29.653 30.167	31.751 31.918 32.952	34.153	35.056 35.619	<b>35.775</b> 36.341	37.171	37.841	38.530 38.558 38.916	39.423 39.511	40.158	40.424 40.630	40.905	41.507 41.939	42.584	42.864 43.014	43.050 43.166	43.251	43.214 43.215	43.265 43.323 43.305	43.369 43.387 43.362	43.275 43.218 43.167	<b>43.096</b> 43.106 43.094	43.024 43.167 43.728	43.382	43.687 43.793 44.226 44.324	44.701 44.701 44.852 44.852 45.131	45.383 45.718 45.722	45.807 45.857 46.185	46.260 46.260 46.367	46.333 46.341 46.400	40.400	46.248	45.952	45.734 45.734	45.415 45.400	45.281	45.301 45.426	45.505	45.010 44.885 <b>44.869</b>
Chainage	0.000 0.006 5.101	10.000	20.000 20.000 30.000 35.000	40.000	000.06	70.000	80.000	100.000	101.676 105.000 110.000	112.452 113.091 120.000	124.312 130.000	140.000 140.906 150.000	160.000	170.000 180.000	182.345 190.000	200.000	210.000	220.000 221.216 230.000	240.000 241.970	260.000	270.000 275.382	280.000	290.000 297.615	310.000	320.000 325.382	330.000 340.000	350.000	358.715 360.000	363.165 370.000 375.382	378.096 380.000 390.000	393.954 400.000 402.407	404.892 410.000 410.741	420.000 427.407 430.000	440.000	450.000 452.407 458.372 460.000	460.246 466.451 470.000 474.558	480.000 487.658 487.800	490.000 491.230 500.000	506.230 510.000 515.423	520.000 521.230 530.000	540.000	550.000	560.000	570.000 580.000	590.000 590.428	600.000	610.000 618.190	620.000 630.000	640.000 650.000 650.233

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Design Levels	22.915 22.804	23.437	23.458	23.769	23.775 24.425	25.075	25.725	26.375	27.025	27.675	28.325	28.805	28.970	29.202 29.506	20 892	2005	30.080 30.127	30.213 30.214	30.149	30.005	29.690	29.399	29.027	28.572	28.037	27.419	26.720	25.980	25.240	
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Chainage	-0.000 5.058	14.792	15.117	19.899	20.000 30.000	40.000	50.000	60.000	70.000	80.000	000.06	97.385	100.000	103.992 110.000	120.000	000.07	127.385	140.000 140.719	150.000	157.385 160.000	170.000	180.000	190.000	200.000	210.000	220.000	230.000	240.000	250.000	



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REV:	ISSUED FOR / DESCRIPTION:



		STATU PRELIMINARY /	IS: INFORMATION	DESIGN BY: RJ DESIGN CHK: JS	rare.		CLIENT: HOLMESDYER	TITLE: LONG SECTION - ROAD 4
		DO NOT SCALE - IF THIS DOCUMENT MAY ONLY BE USED F	IN DOUBT, ASK	DRAWN BY: <b>PVD</b>			PROJECT: BOYER ROAD PRECINCT	
PVD 2	21-05-25	WAS PREPARED. © RARE INNOVATIO	N PTY LTD. ABN 51 619 598 257	DRAFT CHK: -	22-24 Paterson Street	rarein.com.au	ADDRESS: 50 - 170 BOYER ROAD	SCALL AS NOTED STILL SIZE. AT DWGS IN SET
BY:	DATE:	APPROVED: -	ACRED. No: -	DATE: -	Launceston TAS 7250	₽.03 6388 9200	BRIDGEWATER	PROJECT No: 251013 DWG No: C413 REV: A







Α	INFORMATION PURPOSES
REV:	ISSUED FOR / DESCRIPTION:

		42																																			Crest Ch 492.182 RL 24.659	
		Character 2011 Control 12:10 Character 2011 Control 10:12	00					4.2	20%			<	00.02 1.P. 18.828	)	*		5.3	5%			00 I.P. 22.405				*					1.50%								
			$\leq$			t							*	)											ţ					1.00%							$\left  \right $	
-0.454	-0.363	-0.130	-0.087	-0.042	70.01	0.035	-0.120	-0.256	-0.354	-0.137	-0.058	-0.057	+0.187 +0.198	+0.125	+0.114	-0.057	0.023-0.021	-0.065	+0.020	+0.189	+0.112 +0.010	-0.497	-0.913	-0.541	-0.528	-0.349	+0.119	+0.384	+0.372	+0.177	+0.020	-0.196	-0.275	-0.102	+0.143	+0.123	+0.068	-0.105
16.301	16.238	16.144	16.142	10.143	16.230	16.371	16.707	17.127	17.547	17.967	18.387	18.408	18.833 18.856	19.336	19.363	19.871	20.397 20.406	20.532 20.925	21.390	21.801	22.068 22.156	22.457	22.702	22.893	22.930 23.044	23.194	23.344	23.494	23.644	23.794	23.944	24.094	24.244	24.394	24.544	24.655	24.659	24.608
16.755	16.601	16.274	16.229	021.01	16.300	16.406	16.827	17.383	17.901	18.104	18.445	18.465	18.646 18.658	19.211	19.249	19.928	<b>20.420</b> 20.427	20.597 21.057	21.370	21.612	21.956 22.146	22.954	23.615	23.434	23.458 23.727	23.543	23.225	23.110	23.272	23.617	23.924	24.290	24.519	24.496	24.401	24.532	24.591	24.713
192.000	193.997	200.000	201.091	202.000	010 010	212.000	220.000	230.000	240.000	250.000	260.000	260.503	270.000 270.503	280.000	280.503	290.000	299.828 300.000	302.359 310.000	320.000	330.000	337.359 340.000	350.000	360.000	370.000	3/2.359 380.000	390.000	400.000	410.000	420.000	430.000	440.000	450.000	460.000	470.000	480.000	490.000	492.182	500.000
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	STATU: PRELIMINARY / I	s: NFORMATION	DESIGN BY: RJ DESIGN CHK: JS	rdre.			TITLE: LONG SECTION - ROAD 5 & ROAD 6
	DO NOT SCALE - IF I THIS DOCUMENT MAY ONLY BE USED FI WAS PREPARED. © RARE INNOVATION	N DOUBT, ASK OR THE PURPOSE FOR WHICH IT I PTY LTD. ABN 51 619 598 257	DRAWN BY: <b>PVD</b>		•		SCALE: AS NOTED SHEET SIZE: A1 DWGs IN SET: -
PVD         21-05-25           BY:         DATE:	APPROVED: -	ACRED. No: -	DATE: -	22-24 Paterson StreetratLaunceston TAS 7250P.0	<b>rein.com.au</b> 03 6388 9200	BRIDGEWATER	PROJECT No: <b>251013</b> DWG No: <b>C414</b> REV: <b>A</b>









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Α	INFORMATION PURPOSES
REV:	ISSUED FOR / DESCRIPTION:

ROAD 8

		STATU PRELIMINARY /	s: INFORMATION	DESIGN BY: RJ DESIGN CHK: JS	rdre.	CLIENT: HOLMESDYER	TITLE: LONG SECTION - ROAD 7 - ROAD 9
		DO NOT SCALE - IF I THIS DOCUMENT MAY ONLY BE USED F WAS PREPARED. © RARE INNOVATION	IN DOUBT, ASK FOR THE PURPOSE FOR WHICH IT N PTY LTD. ABN 51 619 598 257	DRAWN BY: <b>PVD</b>			SCALE: AS NOTED SHEET SIZE: A1 DWGs IN SET: -
PVD 2 BY:	<b>21-05-25</b> DATE:	APPROVED: -	ACRED. No: -	DATE: -	22-24 Paterson StreetFarein.com.auLaunceston TAS 7250P.03 6388 9200	ADDRESS: 50 - 170 BOYER ROAD BRIDGEWATER	PROJECT NO: 251013 DWG No: C415 REV: A



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/						_	$\square$						-												
+0.179	+0.187	+0.152	+0.142	+0.048	-0.046	-0.068	-0.134	-0.167	-0.168	-0.192	-0.074	+0.012	+0.126	+0.255	+0.125	+0.090	+0.042	+0.088	+0.035	+0.022	000 0.	+0.009	+0.022	+0.034	-0.236
31.823	31.837	31.863	31.873	31.869	31.852	31.720	31.609	31.483	31.235	31.136	30.600	30.323	29.875	28.962	27.954	26.946	25.938	25.807	24.953	24.031	121.00	22.171	22.473	22.373	21.599
31.644	31.650	31.711	31.731	31.821	31.898	31.788	31.743	31.650	31.403	31.328	30.674	30.311	29.749	28.737	27.829	26.856	25.896	25.719	24.918	24.009	11100	22.089	22.451	22.339	21.835
565.000	566.393	570.000	574.123	576.766	580.000	590.000	595.000	600.000	607.633	610.000	620.000	624.137	630.000	640.000	650.000	<u>660.000</u>	670.000	671.292	680.000	000.069		702 109	708.708	710.000	720.000





ROAD 12

Α	INFORMATION PURPOSES
REV:	ISSUED FOR / DESCRIPTION:

-0.018 -0.045 -0.042

16.060 15.810 15.804

16.078 15.855 **15.846** 

160.000 170.000 170.242



ROAD 11







Sad Ch 237.647 RL 9.9															
)							0.80	%							
+0.054	+0.004	+0.002	-0.039	-0.142	-0.216	-0.318	-0.371	-0.395	-0.351	-0.366	-0.371	-0.313	-0.076	+0.002	+0.000
9.976	9.983	9.985	10.065	10.145	10.225	10.305	10.385	10.465	10.545	10.623	10.625	10.645	10.705	10.785	10.829
9.922	9.979	9.983	10.104	10.287	10.441	10.623	10.756	10.860	10.896	10.989	10.996	10.958	10.781	10.783	10.829
237.647	239.673	240.000	250.000	260.000	270.000	280.000	290.000	300.000	310.000	319.645	320.000	322.407	330.000	340.000	345.463

	CLIENT: HOLMESDYER PROJECT: BOYER ROAD PRECINCT	TITLE: LONG SECTION - ROAD 10 - ROAD 14
<b>rarein.com.au</b>	ADDRESS: 50 - 170 BOYER ROAD	SCALE: AS NOTED SHEET SIZE: A1 DWGs IN SET: -
<b>P.</b> 03 6388 9200	BRIDGEWATER	PROJECT No: <b>251013</b> DWG No: <b>C416</b> REV: <b>A</b>



Α	INFORMATION PURPOSES
REV:	ISSUED FOR / DESCRIPTION:

		STATU PRELIMINARY /	STATUS: DESIG		rdre.		CLIENT: HOLMESDYER PROJECT: BOYER ROAD PRECINCT	TITLE: TYPICAL ROAD CROSS SECTIONS
		THIS DOCUMENT MAY ONLY BE USED F WAS PREPARED. © RARE INNOVATIO	FOR THE PURPOSE FOR WHICH IT N PTY LTD. ABN 51 619 598 257	DRAFT CHK: -	22 24 Paterson Street	rarain com au		SCALE: 1:100 SHEET SIZE: A1 DWGs IN SET: -
<b>PVD</b> BY:	21-05-25 DATE:	APPROVED: -	ACRED. No: -	DATE: -	Launceston TAS 7250	<b>P.</b> 03 6388 9200	BRIDGEWATER	PROJECT No: 251013 DWG No: C421 REV: A